Earth Strata Geotechnical Services, Inc.
Geotechnical, Environmental and Materials Testing Consultants

May 23, 2025 Project No. 256070-10A

Mr. David Hensel
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Escondido, CA 92029

Subject: **Preliminary Geotechnical Interpretive Report and Slope Stability Analysis, Proposed Single Family Residential, Assessor's Parcel Number 292-690-06-00, Located at 0 Stonepointe Drive, City of Escondido, San Diego County, California**

Earth Strata Geotechnical Services is pleased to present our preliminary geotechnical interpretive report and slope stability analysis for the proposed single family residence, Assessor's Parcel Number 292-690-06-00, located on Stonepointe Drive in the City of Escondido, San Diego County, California. The purpose of this study is to evaluate the nature, distribution, engineering properties, and geologic strata underlying the site with respect to the proposed development.

Earth Strata Geotechnical Services appreciates the opportunity to offer our consultation and advice on this project. In the event that you have any questions, please do not hesitate to contact the undersigned at your earliest convenience.

Respectfully submitted,

EARTH STRATA GEOTECHNICAL SERVICES

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SMP/mw

TABLE OF CONTENTS

Section **Page**

INTRODUCTION 1
SITE DESCRIPTION 1
PROPOSED DEVELOPMENT AND GRADING 1
FIELD EXPLORATION AND LABORATORY TESTING 3
Field Exploration 3
Laboratory Testing 3
FINDINGS 3
Regional Geology 3
Local Geology 4
Faulting 6
Landslides 6
CONCLUSIONS AND RECOMMENDATIONS 6
General 6
Earthwork 7
Earthwork and Grading 7
Clearing and Grubbing 7
Excavation Characteristics 7
Groundwater 7
Ground Preparation for Fill Areas 7
Oversize Rock 8
Compacted Fill Placement 8
Import Earth Materials 8
Cut/Fill Transitions 9
Cut Areas 10
Shrinkage, Bulking and Subsidence 10
Geotechnical Observations 10
Post Grading Considerations 10
Slope Landscaping and Maintenance 10
Site Drainage 11
Utility Trenches 11
SEISMIC DESIGN CONSIDERATIONS 12
Ground Motions 12
Secondary Seismic Hazards 13
Liquefaction and Lateral Spreading 13
General 13
Allowable Bearing Values 13
Settlement 14
Lateral Resistance 14
Structural Setbacks and Building Clearance 14
Foundation Observations 15
Expansive Soil Considerations 16
Very Low Expansion Potential (Expansion Index of 20 or Less) 16
Footings 16
Building Floor Slabs 16

Corrosivity 17
RETAINING WALLS 18
Active and At-Rest Earth Pressures 18
Subdrain System 18
Temporary Excavations 19
Retaining Wall Backfill 19
CONCRETE FLATWORK 19
Thickness and Joint Spacing 19
Subgrade Preparation 19
SLOPE STABILITY 20
Deep Seated Stability 20
Surficial Stability 20
Slope Stability Results 20
GRADING PLAN REVIEW AND CONSTRUCTION SERVICES 21
REPORT LIMITATIONS 21

Attachments:

Figure 1 – Vicinity Map (Page 2)
Figure 2 – Regional Geologic Map (Page 5)
APPENDIX A – References (Rear of Text)
APPENDIX B – Exploratory Logs (Rear of Text)
APPENDIX C – Laboratory Procedures and Test Results (Rear of Text)
APPENDIX D – Seismicity (Rear of Text)
APPENDIX E - Slope Stability Analysis (Rear of Text)
APPENDIX F – General Earthwork and Grading Specifications (Rear of Text)
Plate 1 – Geotechnical Map (Rear of Text)

INTRODUCTION

Earth Strata Geotechnical Services is pleased to present our preliminary geotechnical interpretive report for the proposed development. The purpose of this study was to evaluate the nature, distribution, engineering properties, and geologic strata underlying the site with respect to the proposed development, and then provide preliminary grading and foundation design recommendations based on the plans you provided. The general location of the subject property is indicated on the Vicinity Map, Figure 1. The plans you provided were used as the base map to show geologic conditions within the subject site, see Geotechnical Map, Plate 1.

SITE DESCRIPTION

The subject property is located on Stonepointe Drive in the City of Escondido, San Diego County, California. The approximate location of the site is shown on the Vicinity Map, Figure 1.

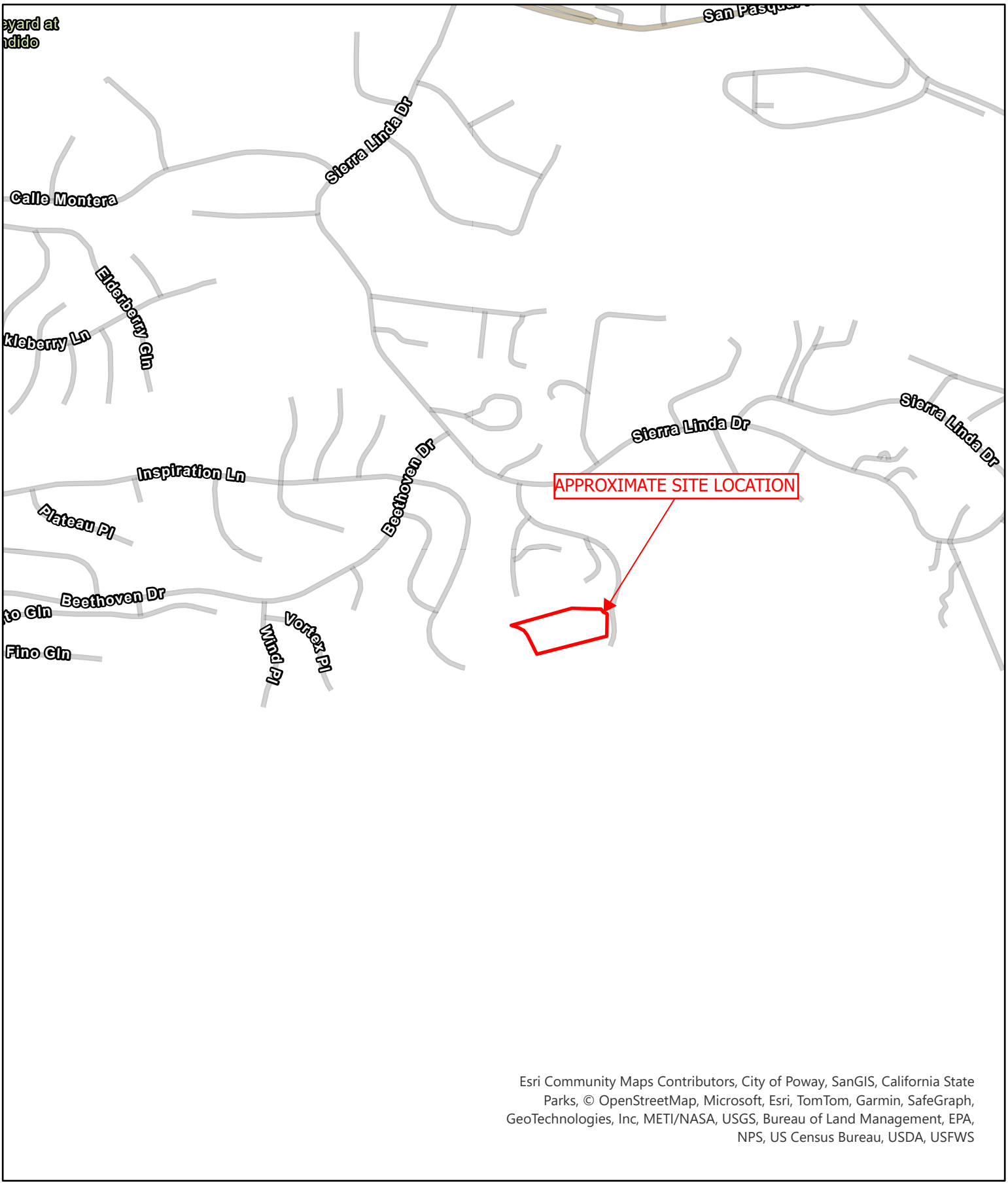
The subject property is comprised of approximately 1.52 acres of undeveloped land. Topographic relief at the subject property is relatively high with the terrain being generally hilly and sloping. Elevations at the site range from approximately 700 to 750 feet above mean sea level (msl), for a difference of about 50± feet across the entire site. Drainage within the subject property generally flows to the northwest.

The site is currently bordered by residential developments as well as vacant property. Most of the vegetation on the site consists of moderate to dense amounts of annual weeds/grasses.

PROPOSED DEVELOPMENT AND GRADING

The proposed residential development is expected to consist of concrete, wood or steel framed one- and/or two-story structures utilizing slab on grade construction with associated streets, landscape areas, and utilities. The current development plans include one (1) building pad positioned throughout the site.

The plans provided by you were utilized in our exploration and form the base for our Geotechnical Map, Plate 1. The plans call for 2:1 (h:v) cut slopes on the order of 15 feet high, 1.5:1 (h:v) cut slopes up to 25 feet high and 2:1 (h:v) fill slopes up to 50 feet high.



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256070-10A FIGURE 1 : VICINITY MAP

280 0 840

Feet

N

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FIELD EXPLORATION AND LABORATORY TESTING

Field Exploration

Subsurface exploration within the subject site was performed in March of 2025 for the exploratory excavations. Hand tools were used to excavate four (4) borings throughout the site to a maximum depth of 4 feet below existing grades.

Earth materials encountered during exploration were classified and logged in general accordance with the Standard Practice for Description and Identification of Soils (Visual-Manual Procedure) of ASTM D 2488. Upon completion of laboratory testing, exploratory logs and sample descriptions may have been reconciled to reflect laboratory test results with regard to ASTM D 2487.

Associated with the subsurface exploration was the collection of bulk (disturbed) samples and relatively undisturbed samples of earth materials for laboratory testing and analysis. The relatively undisturbed samples were obtained with a 3 inch outside diameter modified California split-spoon sampler lined with 1-inch-high brass rings. Samples obtained were mechanically driven, the central portions of the driven samples were placed in sealed containers and transported to our laboratory for testing and analysis. The approximate exploratory locations are shown on Plate 1 and descriptive logs are presented in Appendix B.

Laboratory Testing

Maximum dry density/optimum moisture content, direct shear, expansion potential, pH, resistivity, sulfate content, chloride content, and in-situ density/moisture content were determined for selected undisturbed and bulk samples of earth materials, considered representative of those encountered. An evaluation of the test data is reflected throughout the Conclusions and Recommendations section of this report. A brief description of laboratory test criteria and summaries of test data are presented in Appendix C.

FINDINGS

Regional Geology

Regionally, the site is located in the Peninsular Ranges Geomorphic Province of California. The Peninsular Ranges are characterized by northwest trending steep mountain ranges separated by sediment filled elongated valleys. The dominant structural geologic features reflect the northwest trend of the province. Associated with and subparallel to the San Andreas Fault are the San Jacinto Fault, Newport-Inglewood, and the Whittier-Elsinore Fault. The Santa Ana Mountains abut the west side of the Elsinore Fault while the Perris Block forms the other side of the fault zone to the east. The Perris Block is bounded to the east by the San Jacinto Fault. The northern perimeter of the Los Angeles basin forms part of a northerly dipping blind thrust fault at the boundary between the Peninsular Ranges Province and the Transverse Range Province.

The mountainous regions within the Peninsular Ranges Province are comprised of Pre-Cretaceous, metasedimentary, and metavolcanic rocks along with Cretaceous plutonic rocks of the Southern California Batholith. The low lying areas are primarily comprised of Tertiary and Quaternary non-marine alluvial sediments consisting of alluvial deposits, sandstones, claystones, siltstones, conglomerates, and occasional

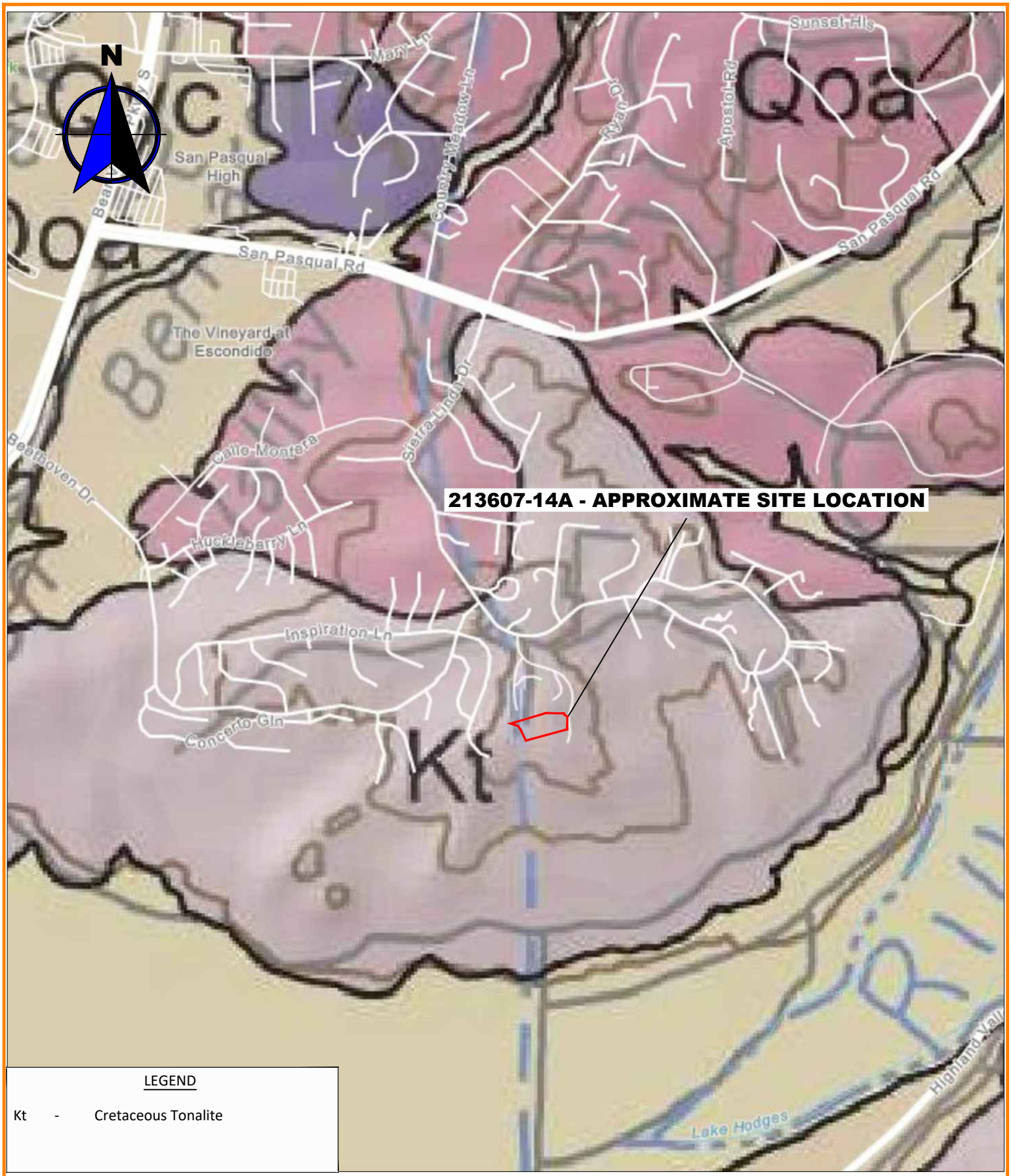
volcanic units. A map illustrating the regional geology is presented on the Regional Geologic Map, Figure 2.

Local Geology

The earth materials on the site are primarily comprised of topsoil and bedrock. A general description of the dominant earth materials observed on the site is provided below:

- Topsoil (no map symbol): Residual topsoil, encountered in the upper 1 to 3 feet, blankets the site and underlying bedrock. These materials were noted to be generally light yellowish brown, sands and silty sands which were porous, dry, and in a loose state.

- Cretaceous Tonalite (map symbol Kt): The tonalite was mapped across the site and was encountered below the topsoil in the subsurface excavations. The tonalite was generally noted to be gray to yellowish brown, dry to slightly moist, and was found to be in a moderately to very hard state. Typically, the upper 1 to 3 feet of this unit is more weathered and not as hard.



REFERENCES: Kennedy, M.P. and Tan, S.S., 2005, Geologic map of the Oceanside 30' x 60' quadrangle, California: A digital database. [superseded by Kennedy, Tan, Bovard, Alvarez, Watson, and Gutierrez, 2007, California Geological Survey Regional Geologic Map No. 2], California Geological Survey, Preliminary Geologic Maps PGM-01-04, 1:100,000.

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REGIONAL GEOLOGIC MAP

SCALE 1:18,056

MAY 2025 | FIGURE 2

Faulting

The project is located in a seismically active region and as a result, significant ground shaking will likely impact the site within the design life of the proposed project. The geologic structure of the entire southern California area is dominated by northwest-trending faults associated with the San Andreas Fault system, which accommodates for most of the right lateral movement associated with the relative motion between the Pacific and North American tectonic plates. Known active faults within this system include the Newport-Inglewood, Whittier-Elsinore, San Jacinto and San Andreas Faults.

No active faults are known to project through the site and the site is not located within an Alquist-Priolo Earthquake Fault Zone, established by the State of California to restrict the construction of new habitable structures across identifiable traces of known active faults. An active fault is defined by the State of California as having surface displacement within the past 11,000 years or during the Holocene geologic time period. Based on our mapping of the subject site, review of current and historical aerial imagery, lack of lineaments indicative of active faulting, and the data compiled during the preparation of this report, it is our interpretation that the potential for surface rupture to adversely impact the proposed structures is very low to remote.

Based on our review of regional geologic maps and applicable computer programs (USGS Seismic Design Maps, Caltrans ARS online, and USGS Earthquake Hazard Programs), the Newport Inglewood Connected alt 1 Fault with an approximate source to site distance of 26.31 kilometers is the closest known active fault anticipated to produce the highest ground accelerations, with an anticipated maximum modal magnitude of 7.50. A list of faults as well as a list of significant historical seismic events within a 100km radius of the subject site are included in Appendix D.

Landslides

Landslide debris was not observed during our subsurface exploration and no ancient landslides are known to exist on the site. No landslides are known to exist, or have been mapped, in the vicinity of the site. Geologic mapping of the site conducted during our investigation, and review of aerial imagery of the site, reveal no geomorphic expressions indicative of landsliding. The materials encountered in the pad area were found to be very hard and no oversteepened slopes exist on the site or are proposed.

CONCLUSIONS AND RECOMMENDATIONS

General

From geotechnical and engineering geologic points of view, the subject property is considered suitable for the proposed development, provided the following conclusions and recommendations are incorporated into the plans and are implemented during construction. The onsite soils are suitable for use as compacted fill.

Earthwork

Earthwork and Grading

The provisions of the 2022 California Building Code (CBC), including the General Earthwork and Grading Specifications in the last Appendix of this report, should be applied to all earthwork and grading operations, as well as in accordance with all applicable grading codes and requirements of the appropriate reviewing agency. Unless specifically revised or amended herein, grading operations should also be performed in accordance with applicable provisions of our General Earthwork and Grading Specifications within the last appendix of this report.

Clearing and Grubbing

Vegetation including trees, grasses, weeds, brush, shrubs, or any other debris should be stripped from the areas to be graded and properly disposed of offsite. In addition, laborers should be utilized to remove any roots, branches, or other deleterious materials during grading operations.

Earth Strata Geotechnical Services should be notified at the appropriate times to provide observation and testing services during Clearing and Grubbing operations. Any buried structures or unanticipated conditions should be brought to our immediate attention.

Excavation Characteristics

Based on the results of our exploration and experience with similar projects in similar settings, the near surface earth materials, will be readily excavated with conventional earth moving equipment. Excavation difficulty is a function of the degree of weathering and amount of fracturing within the bedrock. Bedrock generally becomes harder and more difficult to excavate with increasing depth.

Groundwater

Groundwater was not observed during our subsurface exploration and is not expacted to be a factor during grading.

Subdrain systems should be installed in all canyon areas, buttresses, fill over cut slopes, and/or stabilization fills. The subdrain systems should be installed with a minimum of 10 feet of cover. All subdrain systems should be constructed per the specific guidelines provided within the General Earthwork and Grading Specifications found in the last appendix of this report.

Ground Preparation for Fill Areas

For each area to receive compacted fill, the removal of low density, compressible earth materials, such as topsoil, should continue until firm competent bedrock is encountered. Removal excavations are subject to verification by the project engineer, geologist or their representative. Prior to placing compacted fills, the exposed bottom in each removal area should be scarified to a depth of 6 inches or more, watered or air dried as necessary to achieve near optimum moisture conditions and then compacted to a minimum of 90 percent of the maximum dry density determined by ASTM D 1557.

The intent of remedial grading is to diminish the potential for hydro-consolidation, slope instability, and/or settlement. Remedial grading should extend beyond the perimeter of the proposed structures a horizontal distance equal to the depth of excavation or a minimum of 5 feet, whichever is greater. For cursory purposes the anticipated removal depths are shown on the enclosed Geotechnical Map, Plate 1. In general, the anticipated removal depths should vary from 1 to 3 feet below existing grade.

Wet Removals

Wet alluvial materials will probably not be encountered during construction. If removals of wet alluvial materials are required, special grading equipment and procedures can greatly reduce overall costs. Careful planning by an experienced grading contractor can reduce the need for special equipment, such as swamp cats, draglines, excavators, pumps, and top loading earthmovers. Possible solutions may include the placement of imported angular rock and/or geotextile ground reinforcement. More specific recommendations can be provided based on the actual conditions encountered. Drying or mixing of wet materials with dry materials will be needed to bring the wet materials to near optimum moisture prior to placing wet materials into compacted fills.

Oversize Rock

Oversize rock may be encountered during excavation of deeper cuts during grading. Oversize rock that is encountered (i.e., rock exceeding a maximum dimension of 12 inches) should be disposed of offsite or stockpiled onsite and crushed for future use. The disposal of oversize rock is discussed in greater detail in General Earthwork and Grading Specifications within the last appendix of this report.

Compacted Fill Placement

Compacted fill materials should be placed in 6 to 8 inch maximum (uncompacted) lifts, watered or air dried as necessary to achieve uniform near optimum moisture content and then compacted to a minimum of 90 percent of the maximum dry density determined by ASTM D 1557.

Import Earth Materials

Should import earth materials be needed to achieve final design grades, all potential import materials should be free of deleterious/oversize materials, non-expansive, and approved by the project geotechnical consultant prior to delivery onsite.

Fill Slopes

When properly constructed, fill slopes less than 30 feet high with inclinations of 2:1 (h:v) or flatter are considered to be grossly stable. Keyways are required at the toe of all fill slopes higher than 5 feet and steeper than 5:1 (h:v). Keyways should be a minimum of 10 feet wide and 2 feet into bedrock earth materials, as measured on the downhill side. In order to establish keyway removals, backcuts should be cut no steeper than 1:1 or as recommended by the geotechnical engineer or engineering geologist. Compacted fill should be benched into bedrock earth materials.

Cut Slopes

When properly constructed, cut slopes into bedrock less than 30 feet high with inclinations of 2:1 (h:v) or flatter are considered grossly stable. Cut slopes should be observed by the engineering geologist or his representative during grading, but are anticipated to be stable.

Stabilization Fills

Currently, stabilization fills will not be required for cut slopes in the bedrock. Our engineering geologist or his representative should be called to evaluate all slopes during grading. In the event that unfavorable geologic conditions are encountered, recommendations for stabilization fills or flatter slopes will be provided.

Fill Over Cut Slopes

The fill portion of fill over cut slopes should not be constructed until the cut portion of the slope has been cut to finish grade. The earth materials and geologic structure exposed along the cut slope should be evaluated with regard to suitability for compacted fills or foundations and for stability. If the cut materials are determined to be competent, then the construction of the keyway and subdrain system may commence or additional remedial recommendations will be provided.

Temporary Backcuts

It is the responsibility of the grading contractor to follow all Cal-OSHA requirements with regard to excavation safety. Where existing developments are upslope, adequate slope stability to protect those developments must be maintained. Temporary backcuts will be required to accomplish removals of unsuitable materials and possibly, to perform canyon removals, stabilization fills, and/or keyways. Backcuts should be excavated at a gradient of 1:1 (h:v) or flatter. Flatter backcuts may be required where geologic structure or earth materials are unfavorable. It is imperative that grading schedules minimize the exposure time of the unsupported excavations. All excavations should be stabilized within 30 days of initial excavation.

Cut/Fill Transitions

Cut/fill transitions should be eliminated from all building areas where the depth of fill placed within the "fill" portion exceeds proposed footing depths. This is to diminish distress to structures resulting from excessive differential settlement. The entire foundation of each structure should be founded on a uniform bearing material. This should be accomplished by overexcavating the "cut" portion and replacing the excavated materials as properly compacted fill. Refer to the following table for recommended depths of overexcavation.

DEPTH OF FILL ("fill" portion)	DEPTH OF OVEREXCAVATION ("cut" portion)
Up to 5 feet	Equal Depth
5 to 10 feet	5 feet
Greater than 10 feet	One-half the thickness of fill placed on the "fill" portion (10 feet maximum)

Overexcavation of the "cut" portion should extend beyond the building perimeter a horizontal distance equal to the depth of overexcavation or a minimum of 5 feet, whichever is greater.

Cut Areas

In cut areas, an area a minimum of 5 feet beyond the footprint of the proposed structures should overexcavated until; competent bottoms are achieved; to a minimum 3 feet below the proposed foundations; or per the Overexcavation Table above; (whichever is greater) and replaced with compacted fill. Final determination of areas that require overexcavation should be determined in the field by a representative of Earth Strata Geotechnical Services.

Shrinkage, Bulking and Subsidence

Volumetric changes in earth material quantities will occur when poorly consolidated earth materials are replaced with properly compacted fill. Estimates of the percent shrinkage/bulking factors for the various geologic units observed on the subject property are based on in-place densities and on the estimated average percent of relative compaction achieved during grading.

GEOLOGIC UNIT	SHRINKAGE (%)
Artificial Fill	10 to 15
Bedrock	0 to 5 (bulking)

Subsidence from scarification and recompaction of exposed bottom surfaces is expected to be negligible to approximately 0.01 foot.

The estimates of shrinkage/bulking and subsidence are intended as an aid for project engineers in determining earthwork quantities. Since many variables can affect the accuracy of these estimates, they should be used with caution and contingency plans should be in place for balancing the project.

Geotechnical Observations

Clearing operations, removal of unsuitable materials, and general grading procedures should be observed by the project geotechnical consultant or his representative. No compacted fill should be placed without observations by the geotechnical consultant or his representative to verify the adequacy of the removals.

The project geotechnical consultant or his representative should be present to observe grading operations and to check that minimum compaction requirements and proper lift thicknesses are being met, as well as to verify compliance with the other recommendations presented herein.

Post Grading Considerations

Slope Landscaping and Maintenance

Adequate slope and building pad drainage is essential for the long term performance of the subject site. The gross stability of graded slopes should not be adversely affected, provided all drainage provisions are properly constructed and maintained. Engineered slopes should be landscaped with

deep rooted, drought tolerant maintenance free plant species, as recommended by the project landscape architect.

Site Drainage

Control of site drainage is important for the performance of the proposed project. Roof gutters are recommended for the proposed structures. Pad and roof drainage should be collected and transferred to driveways, adjacent streets, storm-drain facilities, or other locations approved by the building official in non-erosive drainage devices. Drainage should not be allowed to pond on the pad or against any foundation or retaining wall. Drainage should not be allowed to flow uncontrolled over any descending slope. Planters located within retaining wall backfill should be sealed to prevent moisture intrusion into the backfill. Planters located next to structures should be sealed to the depth of the footings. Drainage control devices require periodic cleaning, testing and maintenance to remain effective.

At a minimum, pad drainage should be designed at the minimum gradients required by the CBC. To divert water away from foundations, the ground surface adjacent to foundations should also be graded at the minimum gradients required per the CBC.

Utility Trenches

All utility trench backfill should be compacted at near optimum moisture to a minimum of 90 percent of the maximum dry density determined by ASTM D 1557. For utility trench backfill within pavement areas the upper 6 inches of subgrade materials should be compacted to 95 percent of the maximum dry density determined by ASTM D 1557. This includes within the street right-of-ways, utility easements, under footings, sidewalks, driveways and building floor slabs, as well as within or adjacent to any slopes. Backfill should be placed in approximately 6 to 8 inch maximum loose lifts and then mechanically compacted with a hydro-hammer, rolling with a sheepsfoot, pneumatic tampers, or similar equipment. The utility trenches should be tested by the project geotechnical engineer or their representative to verify minimum compaction requirements are obtained.

In order to minimize the penetration of moisture below building slabs, all utility trenches should be backfilled with compacted fill, lean concrete or concrete slurry where they undercut the perimeter foundation. Utility trenches that are proposed parallel to any building footings (interior and/or exterior trenches), should not be located within a 1:1 (h:v) plane projected downward from the outside bottom edge of the footing.

SEISMIC DESIGN CONSIDERATIONS

Ground Motions

Structures are required to be designed and constructed to resist the effects of seismic ground motions as provided in the 2022 California Building Code Section 1613. The design is dependent on the site class, occupancy category I, II, III, or IV, mapped spectral accelerations for short periods (S_s), and mapped spectral acceleration for a 1-second period (S_1).

In order for structural design to comply with the 2022 CBC, the USGS "US Seismic Design Maps" online tool was used to compile spectral accelerations for the subject property based on data and maps jointly compiled by the United States Geological Survey (USGS) and the California Geological Survey (CGS). The data found in the following table is based on the Maximum Considered Earthquake (MCE) with 5% damped ground motions having a 2% probability of being exceeded in 50 years (2,475 year return period).

The seismic design coefficients were determined by a combination of the site class, mapped spectral accelerations, and occupancy category. The following seismic design coefficients should be implemented during design of the proposed structures. Summaries of the Seismic Hazard Deaggregation graphs and test data are presented in Appendix D.

2022 CBC	FACTOR (ASCE 7-16)
Site Location	Latitude: 33.062775° (North) Longitude: -117.047011°(West)
Site Class	D – Default
Mapped Spectral Accelerations for short periods, S_s	0.857
Mapped Spectral Accelerations for 1-Second Period, S_1	0.314
Maximum Considered Earthquake Spectral Response Acceleration for Short Periods, S_{ms}	1.028
Maximum Considered Earthquake Spectral Response Acceleration for 1-Second Period, S_{m1} (with 50% increase)*	*0.942
Design Spectral Response Acceleration for Short Periods, S_{DS}	0.685
Design Spectral Response Acceleration for 1-Second Period, S_{D1}*	*0.628
Seismic Design Category	D
Importance Factor Based on Occupancy Category	II

*ASCE 7-16/Supplement 3

We performed the probabilistic seismic hazard assessment for the site in accordance with the 2022 CBC, Section 1803.5.11 and 1803.5.12. The probabilistic seismic hazard maps and data files were jointly prepared by the United States Geological Survey (USGS) and the California Geological Survey (CGS) and can be found at the CGS Probabilistic Seismic Hazards Mapping Ground Motion Page. Actual ground shaking intensities at the site may be substantially higher or lower based on complex variables such as the near source directivity effects, depth and consistency of earth materials, topography, geologic structure, direction of fault rupture, and seismic wave reflection, refraction, and attenuation rates. The mean peak ground acceleration was calculated to be 0.453g.

Secondary Seismic Hazards

Secondary effects of seismic shaking considered as potential hazards include several types of ground failure as well as induced flooding. Different types of ground failure, which could occur as a consequence of severe ground shaking at the site, include landslides, ground lurching, shallow ground rupture, and liquefaction/lateral spreading. The probability of occurrence of each type of ground failure depends on the severity of the earthquake, distance from faults, topography, the state of subsurface earth materials, groundwater conditions, and other factors. Based on our experience, subsurface exploration, and laboratory testing, all of the above secondary effects of seismic activity are considered unlikely.

Seismically induced flooding is normally a consequence of a tsunami (seismic sea wave), a seiche (i.e., a wave-like oscillation of surface water in an enclosed basin that may be initiated by a strong earthquake) or failure of a major reservoir or retention system up gradient of the site. Since the site is at an elevation of more than 700 feet above mean sea level and is located more than 14 miles inland from the nearest coastline of the Pacific Ocean, the potential for seismically induced flooding due to a tsunami is considered nonexistent. Since no enclosed bodies of water lie adjacent to or up gradient of the site, the likelihood for induced flooding due to a dam failure or a seiche overcoming the dam's freeboard is considered nonexistent.

Liquefaction and Lateral Spreading

Liquefaction occurs as a result of a substantial loss of shear strength or shearing resistance in loose, saturated, cohesionless earth materials subjected to earthquake induced ground shaking. Potential impacts from liquefaction include loss of bearing capacity, liquefaction related settlement, lateral movements, and surface manifestation such as sand boils. Seismically induced settlement occurs when loose sandy soils become denser when subjected to shaking during an earthquake. The three factors determining whether a site is likely to be subject to liquefaction include seismic shaking, type and consistency of earth materials, and groundwater level. The proposed structures will be supported by compacted fill and competent bedrock, with groundwater at a depth of over 50 feet. As such, the potential for earthquake induced liquefaction and lateral spreading beneath the proposed structures is considered remote due to the recommended compacted fill, lack of shallow groundwater, and the dense nature of the deeper onsite earth materials.

TENTATIVE FOUNDATION DESIGN RECOMMENDATIONS

General

Provided grading is performed in accordance with the recommendations of this report, shallow foundations are considered feasible for support of the proposed structures. Tentative foundation recommendations are provided herein and graphic presentations of relevant recommendations may also be included on the enclosed map.

Allowable Bearing Values

An allowable bearing value of 2,000 pounds per square foot (psf) is recommended for design of 24-inch square pad footings and 12-inch-wide continuous footings founded at a minimum depth of 12 inches below

the lowest adjacent final grade. This value may be increased by 20 percent for each additional 1-foot of width and/or depth to a maximum value of 2,500 psf. Recommended allowable bearing values include both dead and frequently applied live loads and may be increased by one third when designing for short duration wind or seismic forces.

Settlement

Based on the settlement characteristics of the earth materials that underlie the building sites and the anticipated loading, we estimate that the maximum total settlement of the footings will be less than approximately ¾ inch. Differential settlement is expected to be about ½ inch over a horizontal distance of approximately 20 feet, for an angular distortion ratio of 1:480. It is anticipated that the majority of the settlement will occur during construction or shortly after the initial application of loading.

The above settlement estimates are based on the assumption that the grading and construction are performed in accordance with the recommendations presented in this report and that the project geotechnical consultant will observe or test the earth material conditions in the footing excavations.

Lateral Resistance

Passive earth pressure of 250 psf per foot of depth to a maximum value of 2,500 psf may be used to establish lateral bearing resistance for footings. For areas covered with hardscape, passive earth pressure may be taken from the surface. For areas without hardscape, the first 12 inches of the soil profile must be neglected when calculating passive earth pressure. A coefficient of friction of 0.36 times the dead load forces may be used between concrete and the supporting earth materials to determine lateral sliding resistance. The above values may be increased by one-third when designing for short duration wind or seismic forces. When combining passive and friction for lateral resistance, the passive component should be reduced by one third. In no case shall the lateral sliding resistance exceed one-half the dead load for clay, sandy clay, sandy silty clay, silty clay, and clayey silt.

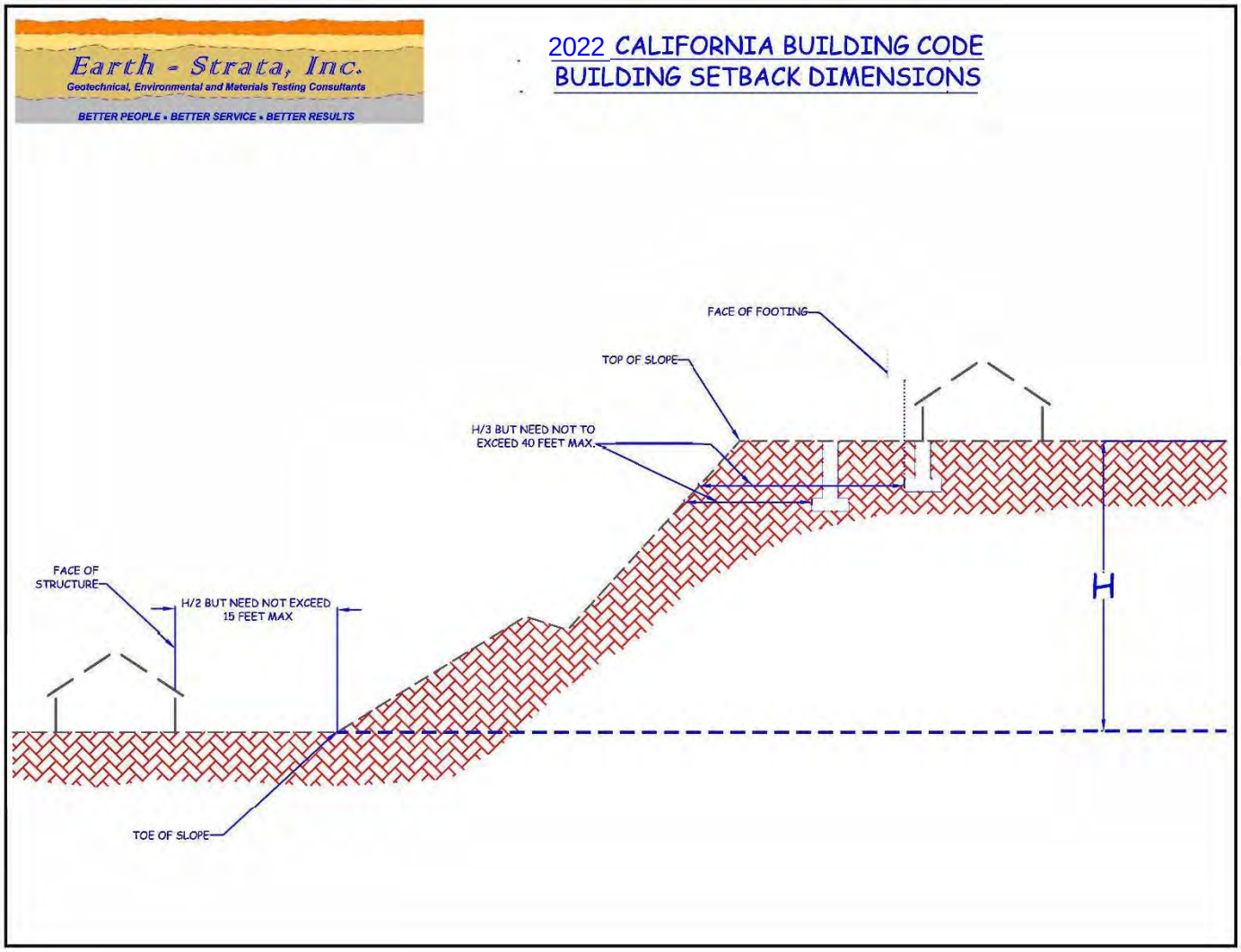
The above lateral resistance values are based on footings for an entire structure being placed directly against either compacted fill or competent bedrock.

Structural Setbacks and Building Clearance

Structural setbacks are required per the 2022 California Building Code (CBC). Additional structural setbacks are not required due to geologic or geotechnical conditions within the site. Improvements constructed in close proximity to natural or properly engineered and compacted slopes can, over time, be affected by natural processes including gravity forces, weathering, and long term secondary settlement. As a result, the CBC requires that buildings and structures be setback or footings deepened to resist the influence of these processes.

For structures that are planned near ascending and descending slopes, the footings should be embedded to satisfy the requirements presented in the CBC, Section 1808.7 as illustrated in the following Foundation Clearances from Slopes diagram.

FOUNDATION CLEARANCES FROM SLOPES



When determining the required clearance from ascending slopes with a retaining wall at the toe, the height of the slope shall be measured from the top of the wall to the top of the slope. The structural setback for pools may be reduced by one-half.

Foundation Observations

In accordance with the 2022 CBC and prior to the placement of forms, concrete, or steel, all foundation excavations should be observed by the geologist, engineer, or his representative to verify that they have been excavated into competent bearing materials. The excavations should be per the approved plans, moistened, cleaned of all loose materials, trimmed neat, level, and square. Any moisture softened earth materials should be removed prior to steel or concrete placement.

Earth materials from foundation excavations should not be placed in slab on grade areas unless the materials are tested for expansion potential and compacted to a minimum of 90 percent of the maximum dry density.

Expansive Soil Considerations

Preliminary laboratory test results indicate onsite earth materials exhibit an expansion potential of **VERY LOW** as classified in accordance with 2022 CBC Section 1803.5.3 and ASTM D 4829. Additional, testing for expansive soil conditions should be conducted upon completion of rough grading. The following recommendations should be considered the very minimum requirements, for the earth materials tested. It is common practice for the project architect or structural engineer to require additional slab thickness, footing sizes, and/or reinforcement.

Very Low Expansion Potential (Expansion Index of 20 or Less)

Our laboratory test results indicate that the earth materials onsite exhibit a **VERY LOW** expansion potential as classified in accordance with 2022 CBC Section 1803.5.3 and ASTM D 4829. Since the onsite earth materials exhibit expansion indices of 20 or less, the design of slab on ground foundations is exempt from the procedures outlined in Section 1808.6.1 or 1808.6.2.

Footings

- Exterior continuous footings may be founded at the minimum depths below the lowest adjacent final grade (i.e. 12-inch minimum depth for one-story, 18-inch minimum depth for two-story, and 24-inch minimum depth for three-story construction). Interior continuous footings for one-, two-, and three-story construction may be founded at a minimum depth of 12 inches below the lowest adjacent final grade. All continuous footings should have a minimum width of 12, 15, and 18 inches, for one-, two-, and three-story structures, respectively per Table 1809.7 of the 2022 CBC, and should be reinforced with a minimum of two (2) No. 4 bars, one (1) top and one (1) bottom.

- Exterior pad footings intended to support roof overhangs, such as second story decks, patio covers and similar construction should be a minimum of 24 inches square and founded at a minimum depth of 18 inches below the lowest adjacent final grade. No special reinforcement of the pad footings will be required.

Building Floor Slabs

- Building floor slabs should be a minimum of 4 inches thick and reinforced with a minimum of No. 3 bars spaced a maximum of 24 inches on center, each way. All floor slab reinforcement should be supported on concrete chairs or bricks to ensure the desired placement at mid-depth.

- Interior floor slabs, within living or moisture sensitive areas, should be underlain by a minimum 10-mil thick moisture/vapor barrier to help reduce the upward migration of moisture from the underlying earth materials. The moisture/vapor barrier used should meet the performance standards of an ASTM E 1745 Class A material, and be properly installed in accordance with Cal Green Standard 4.505.2. It is the responsibility of the contractor to ensure that the moisture/vapor barriers are free of openings, rips, or punctures prior to placing concrete. As an option for additional moisture reduction, higher strength concrete, such as a minimum 28-day compressive strength of 5,000 pounds per square inch (psi) may be used. Ultimately, the design

of the moisture/vapor barrier system and recommendations for concrete placement and curing are the purview of the foundation engineer, taking into consideration the project requirements provided by the architect and owner.

- Garage floor slabs should be a minimum of 4 inches thick and should be reinforced in a similar manner as living area floor slabs. Garage floor slabs should be placed separately from adjacent wall footings with a positive separation maintained with ⅜ inch minimum felt expansion joint materials and quartered with weakened plane joints. A 12-inch-wide turn down founded at the same depth as adjacent footings should be provided across garage entrances. The turn down should be reinforced with a minimum of two (2) No. 4 bars, one (1) top and one (1) bottom.

- The subgrade earth materials below all floor slabs should be pre-watered to promote uniform curing of the concrete and minimize the development of shrinkage cracks, prior to placing concrete. The pre-watering should be verified by Earth Strata Geotechnical Services during construction.

Corrosivity

Corrosion is defined by the National Association of Corrosion Engineers (NACE) as "a deterioration of a substance or its properties because of a reaction with its environment." From a geotechnical viewpoint, the "substances" are the reinforced concrete foundations or buried metallic elements (not surrounded by concrete) and the "environment" is the prevailing earth materials in contact with them. Many factors can contribute to corrosivity, including the presence of chlorides, sulfates, salts, organic materials, different oxygen levels, poor drainage, different soil types, and moisture content. It is not considered practical or realistic to test for all of the factors which may contribute to corrosivity.

The potential for concrete exposure to chlorides is based upon the recognized Caltrans reference standard "Bridge Design Specifications", under Subsection 8.22.1 of that document, Caltrans has determined that "Corrosive water or soil contains more than 500 parts per million (ppm) of chlorides". Based on limited preliminary laboratory testing, the onsite earth materials have chloride contents *less* than 500 ppm. As such, specific requirements resulting from elevated chloride contents are not required.

Specific guidelines for concrete mix design are provided in 2022 CBC Section 1904.1 and ACI 318-19, Section 4.3 Tables 19.3.1.1 and 19.3.2.1 when the soluble sulfate content of earth materials exceeds 0.1 percent by weight. Based on limited preliminary laboratory testing, the onsite earth materials are classified in accordance with Tables 19.3.1.1 and 19.3.2.1 as having a *negligible* sulfate exposure condition. Therefore, structural concrete in contact with onsite earth materials should utilize Type I or II.

Based on our laboratory testing of resistivity, the onsite earth materials in contact with buried steel should be considered possess *moderate* corrosion potential. Soil pH values below 5.6 and above 9.1 are recognized as being corrosive to many common metallic components; the pH values for the earth materials tested were *lower* than 9.1 and *higher* than 5.6, thus soils are not considered corrosive from pH.

The preliminary test results for corrosivity are based on limited samples, and the initiation of grading may blend various earth materials together. This blending or imported material could alter and increase the detrimental properties of the onsite earth materials. Accordingly, additional testing for chlorides and

sulfates along with testing for pH and resistivity should be performed upon completion of grading. Laboratory test results are presented in Appendix C.

RETAINING WALLS

Active and At-Rest Earth Pressures

Foundations may be designed in accordance with the recommendations provided in the Tentative Foundation Design Recommendation section of this report. The following table provides the minimum recommended equivalent fluid pressures for design of retaining walls a maximum of 6 feet high. The active earth pressure should be used for design of unrestrained retaining walls, which are free to tilt slightly. The at-rest earth pressure should be used for design of retaining walls that are restrained at the top, such as basement walls, curved walls with no joints, or walls restrained at corners. For curved walls, active pressure may be used if tilting is acceptable and construction joints are provided at each angle point and at a minimum of 15 foot intervals along the curved segments.

MINIMUM STATIC EQUIVALENT FLUID PRESSURES (pcf)		
PRESSURE TYPE	BACKSLOPE CONDITION	
	LEVEL	2:1 (h:v)
Active Earth Pressure	40	63
At-Rest Earth Pressure	60	95

The retaining wall parameters provided do not account for hydrostatic pressure behind the retaining walls. Therefore, the subdrain system is a very important part of the design. All retaining walls should be designed to resist surcharge loads imposed by other nearby walls, structures, or vehicles should be added to the above earth pressures, if the additional loads are being applied within a 1.5:1 (h:v) plane projected up from the heel of the retaining wall footing. As a way of minimizing surcharge loads and the settlement potential of nearby buildings, the footings for the building can be deepened below the 1.5:1 (h:v)plane projected up from the heel of the retaining wall footing.

Upon request and under a separate scope of work, more detailed analyses can be performed to address equivalent fluid pressures with regard to stepped retaining walls, actual retaining wall heights, actual backfill inclinations, specific backfill materials, higher retaining walls requiring earthquake design motions, etc.

Subdrain System

We recommend a perforated pipe and gravel subdrain system be provided behind all proposed retaining walls to prevent the buildup of hydrostatic pressure behind the proposed retaining walls. The perforated pipe should consist of 4-inch minimum diameter Schedule 40 PVC or ABS SDR-35, placed with the perforations facing down. The pipe should be surrounded by 1 cubic foot per foot of ¾- or 1½ inch open graded gravel wrapped in filter fabric. The filter fabric should consist of Mirafi 140N or equivalent to prevent infiltration of fines and subsequent clogging of the subdrain system.

In lieu of a perforated pipe and gravel subdrain system, weep holes or open vertical masonry joints may be provided in the lowest row of block exposed to the air to prevent the buildup of hydrostatic pressure

behind the proposed retaining walls. Weep holes should be a minimum of 3 inches in diameter and provided at intervals at least every 6 feet along the wall. Open vertical masonry joints should be provided at a minimum of 32 inch intervals. A continuous gravel fill, a minimum of 1 cubic foot per foot, should be placed behind the weep holes or open masonry joints. The gravel should be wrapped in filter fabric consisting of Mirafi 140N or equivalent.

The retaining walls should be adequately coated on the backfilled side of the walls with a proven waterproofing compound by an experienced professional to inhibit infiltration of moisture through the walls.

Temporary Excavations

All excavations should be made in accordance with Cal-OSHA requirements. Earth Strata Geotechnical Services is not responsible for job site safety.

Retaining Wall Backfill

Retaining wall backfill materials should be approved by the geotechnical engineer or his representative prior to placement as compacted fill. Retaining wall backfill should be placed in lifts no greater than 6 to 8 inches, watered or air dried as necessary to achieve near optimum moisture contents. All retaining wall backfill should be compacted to a minimum of 90 percent of the maximum dry density as determined by ASTM D 1557. Retaining wall backfill should be capped with a paved surface drain.

CONCRETE FLATWORK

Thickness and Joint Spacing

Concrete sidewalks and patio type slabs should be at least 3½ inches thick and provided with construction or expansion joints every 6 feet or less, to reduce the potential for excessive cracking. Concrete driveway slabs should be at least 4 inches thick and provided with construction or expansion joints every 10 feet or less.

Subgrade Preparation

In order to reduce the potential for unsightly cracking, subgrade earth materials underlying concrete flatwork should be compacted at near optimum moisture to a minimum of 90 percent of the maximum dry density determined by ASTM D 1557 and then moistened to optimum or slightly above optimum moisture content. This moisture should extend to a depth of 12 inches below subgrade and be maintained prior to placement of concrete. Pre-watering of the earth materials prior to placing concrete will promote uniform curing of the concrete and minimize the development of shrinkage cracks. The project geotechnical engineer or his representative should verify the density and moisture content of the earth materials and the depth of moisture penetration prior to placing concrete.

Cracking within concrete flatwork is often a result of factors such as the use of too high a water to cement ratio and/or inadequate steps taken to prevent moisture loss during the curing of the concrete. Concrete

distress can be reduced by proper concrete mix design and proper placement and curing of the concrete. Minor cracking within concrete flatwork is normal and should be expected.

SLOPE STABILITY

Significance of Slope Stability Analyses

Limit equilibrium analyses of slope stability only provide a general indication of the relative stability of a slope. The applicability is highly dependent on the ability of its simplified analytical methodology and the chosen generalized assumptions of soil properties and slope geometry, to accurately model complex geologic conditions that exist in the field. However, in spite of the limitations, limit equilibrium slope stability analyses can be used to provide insight into the relative need for and benefit of slope stabilization measures.

Deep Seated Stability

Deep seated stability of fill slopes with heights greater than 30 feet or cut slopes steeper than 2:1, were evaluated using Slope/W, a computer application of the Morgenstern Price Method of analysis. Based on our current understanding of the project, slopes on the subject property requiring analysis include a 50 foot high 2:1 (h:v) fill slope, as well as 1.5:1 (h:v) cut slopes up to 25 feet high.

The factor of safety of the slopes was evaluated under both static and pseudostatic loading conditions. Per local code requirements, the minimum acceptable factor of safety was taken as 1.5 for static loading and 1.1 for pseudostatic loading. The pseudostatic analysis includes the effects of static loads combined with horizontal inertial force acting out of slope and through the center of gravity of the potential sliding mass.

Surficial Stability

The surficial stability of near surface earth materials can be calculated using an infinite slope with seepage occurring parallel to the slope face. In the analysis, the vertical depth of saturation is a minimum of 4 feet, and per local code requirements, the minimum acceptable factor of safety for surficial stability is 1.5 for static loading conditions. Our calculations indicate that compacted fill slopes constructed at a 2:1 (h:v) or flatter are surficially stable with a factor of safety greater than 1.5, see Surficial Stability – Calculation Sheet No. 1, enclosed herein.

For cut slopes into bedrock, the surficial earth materials are removed when the cut slopes are constructed. Therefore, the parallel seepage model for calculating surficial stability is not applicable for cut slopes, as the surficial materials have been removed.

Slope Stability Results

The slope stability analyses performed for the sections analyzed on this project indicate that the static factors of safety for potential deep seated slip surfaces are above 1.5 for static and 1.1 for dynamic conditions. The slope stability results are presented in the table below and calculation sheets are presented within the appendices of this report.

CALCULATED FACTORS OF SAFETY

SLOPE TYPE	HEIGHT (ft)	STATIC	PSEUDOSTATIC	SURFICIAL
Cross Section A-A' 2:1 Compacted Fill Slope	50	2.929	1.916	2.49
Cross Section B-B' 1.5:1 Cut Slope	25	3.187	1.996	N/A

An Earth Strata Geotechnical Services geologist should evaluate all slopes in the field during grading and construction. If unfavorable geological conditions are observed or encountered, then stabilization fills or flatter slopes may be required. Any stabilization fills should be constructed per the recommendations herein. All fill slope construction should be properly keyed and benched into competent earth materials.

GRADING PLAN REVIEW AND CONSTRUCTION SERVICES

This report has been prepared for the exclusive use of **Mr. David Hensel** and their authorized representative. It likely does not contain sufficient information for other parties or other uses. Earth Strata Geotechnical Services should be engaged to review the final design plans and specifications prior to construction. This is to verify that the recommendations contained in this report have been properly incorporated into the project plans and specifications. Should Earth Strata Geotechnical Services not be accorded the opportunity to review the project plans and specifications, we are not responsibility for misinterpretation of our recommendations.

We recommend that Earth Strata Geotechnical Services be retained to provide geologic and geotechnical engineering services during grading and foundation excavation phases of the work. In order to allow for design changes in the event that the subsurface conditions differ from those anticipated prior to construction.

Earth Strata Geotechnical Services should review any changes in the project and modify and approve in writing the conclusions and recommendations of this report. This report and the drawings contained within are intended for design input purposes only and are not intended to act as construction drawings or specifications. In the event that conditions encountered during grading or construction operations appear to be different than those indicated in this report, this office should be notified immediately, as revisions may be required.

REPORT LIMITATIONS

Our services were performed using the degree of care and skill ordinarily exercised, under similar circumstances, by reputable soils engineers and geologists, practicing at the time and location this report was prepared. No other warranty, expressed or implied, is made as to the conclusions and professional advice included in this report.

Earth materials vary in type, strength, and other geotechnical properties between points of observation and exploration. Groundwater and moisture conditions can also vary due to natural processes or the works of man on this or adjacent properties. As a result, we do not and cannot have complete knowledge of the

subsurface conditions beneath the subject property. No practical study can completely eliminate uncertainty with regard to the anticipated geotechnical conditions in connection with a subject property. The conclusions and recommendations within this report are based upon the findings at the points of observation and are subject to confirmation by Earth Strata Geotechnical Services based on the conditions revealed during grading and construction.

This report was prepared with the understanding that it is the responsibility of the owner or their representative, to ensure that the conclusions and recommendations contained herein are brought to the attention of the other project consultants and are incorporated into the plans and specifications. The owners' contractor should properly implement the conclusions and recommendations during grading and construction, and notify the owner if they consider any of the recommendations presented herein to be unsafe or unsuitable.

APPENDIX A

REFERENCES

APPENDIX A

References

California Building Standards Commission, 2022, *2022 California Building Code, California Code of Regulations Title 24, Part 2, Volume 2 of 2,* Based on 2018 International Building Code.

California Corrosion Guidelines

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Hart, Earl W. and Bryant, William A., *1997, Fault Rupture Hazard Zones in California, CDMG Special Publication 42,* revised 2003.

Jenkins, Olaf P., 1978, *Geologic Map of California, Santa Ana Sheet*; CDMG, Scale 1:250,000.

Kennedy, M.P., 2000, Nelson, B., and R. Hauser, *Geologic Map of the Pechanga 7.5 Minute Quadrangle, Riverside and San Diego Counties, California, Version 1.0*: U.S. Geological Survey, CDMG.

Kennedy, M.P., 1977, *Regency and Character of Faulting Along the Elsinore Fault Zone in Southern Riverside County, California*, California Division of Mines and Geology Special Report 131.

Morton, D.M., Hauser, Rachel M., and Ruppert, Kelly R., 2004, *Preliminary Digital Geologic Map of the Murrieta 7.5 Minute Quadrangle, Southern California, Version 1.0*: U.S. Geological Survey Open-File Report 99-0172.

National Bureau of Standards, 1989, Underground corrosion Circular 579.

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Southern California Earthquake Center (SCEC), 1999, *Recommended Procedures for Implementation of DMG Special Publication 117, Guidelines for Analyzing and Mitigating Liquefaction Hazards in California,* March.

APPENDIX B

EXPLORATORY LOGS

Geotechnical Log B-1

Date: 5/16/25	**Project Name: Stonepointe Dr** **Page: 1 of 1**
Project Number: 256070-10A	**Logged By: JW**
Drilling Company: ESGS	**Type of Rig: Hand Tools**
Drive Weight (lbs): N/A	**Drop (in): N/A** **Hole Diameter (in): 4**
Top of Hole Elevation (ft): See Map	**Hole Location: See Geotechnical Map**

Depth (ft)	Blow Count Per Foot	Sample Depth	Dry Density (pcf)	Moisture (%)	Classification Symbol	MATERIAL DESCRIPTION
0						**Topsoil:**
		1.5	117.7	1.8		Silty SAND; yellowish brown, dry, loose
						Cretaceous Tonalite (Kt):
						TONALITE; yellowish brown, dry, hard, medium grained, practical refusal @ 3'
						Total Depth = 3 feet
5						No Groundwater
10						
15						
20						
25						
30						

42184 Remington Avenue, Temecula, CA 92590

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Geotechnical, Environmental and Materials Testing Consultants
www.ESGSINC.com (951) 397-8315

Geotechnical Log B-2

Date: 5/16/25	**Project Name: Stonepointe Dr** **Page: 1 of 1**
Project Number: 256070-10A	**Logged By: JW**
Drilling Company: ESGS	**Type of Rig: Hand Tools**
Drive Weight (lbs): N/A	**Drop (in): N/A** **Hole Diameter (in): 4**
Top of Hole Elevation (ft): See Map	**Hole Location: See Geotechnical Map**

Depth (ft)	Blow Count Per Foot	Sample Depth	Dry Density (pcf)	Moisture (%)	Classification Symbol	MATERIAL DESCRIPTION
0						**Topsoil:**
						Silty SAND; yellowish brown, dry, loose
		2	126.1	1.4		**Cretaceous Tonalite (Kt):**
						TONALITE; yellowish brown, dry, hard, medium grained, practical refusal @ 4'
						Total Depth = 4 feet
5						No Groundwater
10						
15						
20						
25						
30						

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Geotechnical Log B-3

Date: 5/16/25	**Project Name: Stonepointe Dr** **Page: 1 of 1**
Project Number: 256070-10A	**Logged By: JW**
Drilling Company: ESGS	**Type of Rig: Hand Tools**
Drive Weight (lbs): N/A	**Drop (in): N/A** **Hole Diameter (in): 4**
Top of Hole Elevation (ft): See Map	**Hole Location: See Geotechnical Map**

Depth (ft)	Blow Count Per Foot	Sample Depth	Dry Density (pcf)	Moisture (%)	Classification Symbol	MATERIAL DESCRIPTION
0		0.5	128.9	1.3		**Cretaceous Tonalite (Kt):**
						TONALITE; yellowish brown, dry, hard, medium grained, practical refusal @ 2'
						Total Depth = 2'
						No Groundwater
5						
10						
15						
20						
25						
30						

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Geotechnical Log B-4

Date: 5/16/25	**Project Name: Stonepointe Dr** **Page: 1 of 1**
Project Number: 256070-10A	**Logged By: JW**
Drilling Company: ESGS	**Type of Rig: Hand Tools**
Drive Weight (lbs): N/A	**Drop (in): N/A** **Hole Diameter (in): 4**
Top of Hole Elevation (ft): See Map	**Hole Location: See Geotechnical Map**

Depth (ft)	Blow Count Per Foot	Sample Depth	Dry Density (pcf)	Moisture (%)	Classification Symbol	MATERIAL DESCRIPTION
0						**Topsoil:**
		1	100.7	1.2		Silty SAND; yellowish brown, dry, loose
						Cretaceous Tonalite (Kt):
						TONALITE; yellowish brown, dry, hard, medium grained, practical refusal @ 3'
						Total Depth = 3 feet
5						No Groundwater
10						
15						
20						
25						
30						

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APPENDIX C

LABORATORY PROCEDURES AND TEST RESULTS

APPENDIX C

Laboratory Procedures and Test Results

Laboratory testing provided quantitative and qualitative data involving the relevant engineering properties of the representative earth materials selected for testing. The representative samples were tested in general accordance with American Society for Testing and Materials (ASTM) procedures and/or California Test Methods (CTM).

Soil Classification: Earth materials encountered during exploration were classified and logged in general accordance with the Standard Practice for Description and Identification of Soils (Visual-Manual Procedure) of ASTM D 2488. Upon completion of laboratory testing, exploratory logs and sample descriptions were reconciled to reflect laboratory test results with regard to ASTM D 2487.

Moisture and Density Tests: For select samples moisture content was determined using the guidelines of ASTM D 2216 and dry density determinations were made using the guidelines of ASTM D 2937. These tests were performed on relatively undisturbed samples and the test results are presented on the exploratory logs.

Maximum Density Tests: The maximum dry density and optimum moisture content of representative samples were determined using the guidelines of ASTM D 1557. The test results are presented in the table below.

SAMPLE LOCATION	MATERIAL DESCRIPTION	MAXIMUM DRY DENSITY (pcf)	OPTIMUM MOISTURE CONTENT (%)
Bulk	Silty SAND	131.5	8.5

Expansion Index: The expansion potential of representative samples was evaluated using the guidelines of ASTM D 4829. The test results are presented in the table below.

SAMPLE LOCATION	MATERIAL DESCRIPTION	EXPANSION INDEX	EXPANSION POTENTIAL
B-1 @ 0 to 3	Silty SAND	1	Very Low

Direct Shear: Direct shear tests were performed on representative remolded and/or undisturbed samples using the guidelines of ASTM D 3080. The test results are presented in the table below.

SAMPLE LOCATION	MATERIAL DESCRIPTION	FRICTION ANGLE (degrees)	APPARENT COHESION (psf)
B-1 @ 0 to 3 (Remolded)*	Silty SAND	38.1	355
B-2 @ 2' (Undisturbed)	Silty SAND	35	950

*Remolded to 90 percent of the maximum dry density.

Minimum Resistivity and pH Tests: Minimum resistivity and pH Tests of select samples were performed using the guidelines of CTM 643. The test results are presented in the table below.

SAMPLE LOCATION	MATERIAL DESCRIPTION	pH	MINIMUM RESISTIVITY (ohm-cm)
B-1 @ 0 to 3	Silty SAND	7.39	9411

Soluble Sulfate: The soluble sulfate content of select samples was determined using the guidelines of CTM 417. The test results are presented in the table below.

SAMPLE LOCATION	MATERIAL DESCRIPTION	SULFATE CONTENT (% by weight)	SULFATE EXPOSURE
B-1 @ 0 to 3	Silty SAND	0.002	Negligible

Chloride Content: Chloride content of select samples was determined using the guidelines of CTM 422. The test results are presented in the table below.

SAMPLE LOCATION	MATERIAL DESCRIPTION	CHLORIDE CONTENT (ppm)
B-1 @ 0 to 3	Silty SAND	30

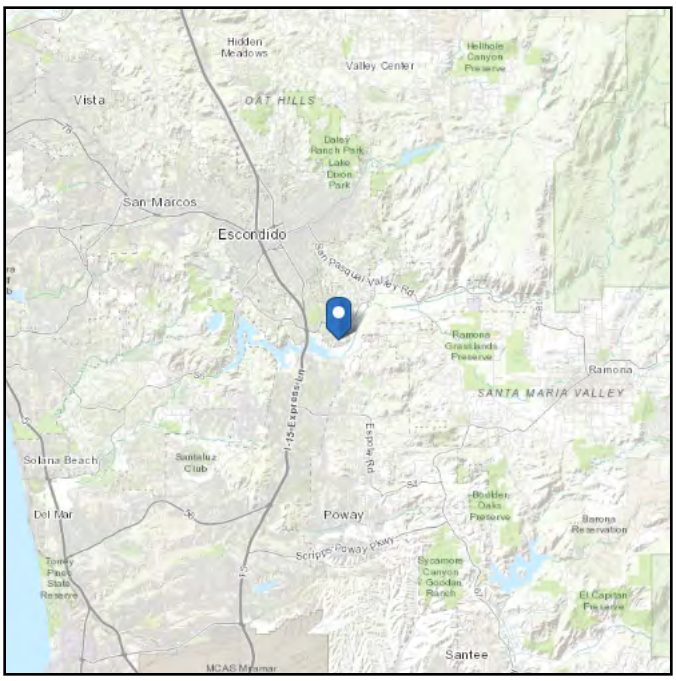
APPENDIX D

SEISMICITY

ASCE Hazards Report

Address:
No Address at This Location

Standard:	ASCE/SEI 7-16	**Latitude:**	33.062775
Risk Category:	II	**Longitude:**	-117.047011
Soil Class:	D - Default (see Section 11.4.3)	**Elevation:**	747.3702056612527 ft (NAVD 88)



Seismic

Site Soil Class: D - Default (see Section 11.4.3)

Results:

S_S :	0.857	S_{D1} :	N/A
S_1 :	0.314	T_L :	8
F_a :	1.2	PGA :	0.368
F_v :	N/A	PGA_M :	0.453
S_{MS} :	1.028	F_{PGA} :	1.232
S_{M1} :	N/A	I_e :	1
S_{DS} :	0.685	C_v :	1.228

Ground motion hazard analysis may be required. See ASCE/SEI 7-16 Section 11.4.8.

Data Accessed: Fri Apr 25 2025

Date Source: USGS Seismic Design Maps

The ASCE Hazard Tool is provided for your convenience, for informational purposes only, and is provided “as is” and without warranties of any kind. The location data included herein has been obtained from information developed, produced, and maintained by third party providers; or has been extrapolated from maps incorporated in the ASCE standard. While ASCE has made every effort to use data obtained from reliable sources or methodologies, ASCE does not make any representations or warranties as to the accuracy, completeness, reliability, currency, or quality of any data provided herein. Any third-party links provided by this Tool should not be construed as an endorsement, affiliation, relationship, or sponsorship of such third-party content by or from ASCE.

ASCE does not intend, nor should anyone interpret, the results provided by this Tool to replace the sound judgment of a competent professional, having knowledge and experience in the appropriate field(s) of practice, nor to substitute for the standard of care required of such professionals in interpreting and applying the contents of this Tool or the ASCE standard.

In using this Tool, you expressly assume all risks associated with your use. Under no circumstances shall ASCE or its officers, directors, employees, members, affiliates, or agents be liable to you or any other person for any direct, indirect, special, incidental, or consequential damages arising from or related to your use of, or reliance on, the Tool or any information obtained therein. To the fullest extent permitted by law, you agree to release and hold harmless ASCE from any and all liability of any nature arising out of or resulting from any use of data provided by the ASCE Hazard Tool.

U.S. Geological Survey - Earthquake Hazards Program

2008 National Seismic Hazard Maps - Source Parameters

New Search

Distance in Kilometers	Name	State	Pref Slip Rate (mm/yr)	Dip (degrees)	Dip Dir	Slip Sense	Rupture Top (km)	Rupture Bottom (km)	Length (km)
26.31	Newport Inglewood Connected alt 1	CA	1.3	89		strike slip	0	11	208
26.31	Newport Inglewood Connected alt 2	CA	1.3	90	V	strike slip	0	11	208
26.31	Rose Canyon	CA	1.5	90	V	strike slip	0	8	70
30.23	Elsinore;GI+T+J+CM	CA	n/a	86	NE	strike slip	0	16	195
30.23	Elsinore;GI+T+J	CA	n/a	86	NE	strike slip	0	17	153
30.23	Elsinore;T+J	CA	n/a	86	NE	strike slip	0	17	127
30.23	Elsinore;W+GI+T+J+CM	CA	n/a	84	NE	strike slip	0	16	241
30.23	Elsinore;J+CM	CA	3	84	NE	strike slip	0	17	118
30.23	Elsinore;J	CA	3	84	NE	strike slip	0	19	75
30.23	Elsinore;T+J+CM	CA	n/a	85	NE	strike slip	0	16	169
30.23	Elsinore;W+GI+T+J	CA	n/a	84	NE	strike slip	0	16	199
31.05	Elsinore;T	CA	5	90	V	strike slip	0	14	52
31.05	Elsinore;GI+T	CA	5	90	V	strike slip	0	14	78

31.05	Elsinore;W+GI+T	CA	n/a	84	NE	strike slip	0	14	124
37.21	Newport-Inglewood (Offshore)	CA	1.5	90	V	strike slip	0	10	66
44.90	Earthquake Valley	CA	2	90	V	strike slip	0	19	20
49.41	Palos Verdes Connected	CA	3	90	V	strike slip	0	10	285
49.41	Coronado Bank	CA	3	90	V	strike slip	0	9	186
65.01	San Jacinto;SJV+A+CC+B	CA	n/a	90	V	strike slip	0.1	15	170
65.01	San Jacinto;CC	CA	4	90	V	strike slip	0	16	43
65.01	San Jacinto;CC+B	CA	n/a	90	V	strike slip	0.2	14	77
65.01	San Jacinto;CC+B+SM	CA	n/a	90	V	strike slip	0.2	14	103
65.01	San Jacinto;A+CC+B+SM	CA	n/a	90	V	strike slip	0.1	15	178
65.01	San Jacinto;A+CC+B	CA	n/a	90	V	strike slip	0.1	15	152
65.01	San Jacinto;A+CC	CA	n/a	90	V	strike slip	0	16	118
65.01	San Jacinto;SBV+SJV+A+CC	CA	n/a	90	V	strike slip	0	16	181
65.01	San Jacinto;SBV+SJV+A+CC+B	CA	n/a	90	V	strike slip	0.1	15	215
65.01	San Jacinto;SBV+SJV+A+CC+B+SM	CA	n/a	90	V	strike slip	0.1	15	241
65.01	San Jacinto;SJV+A+CC+B+SM	CA	n/a	90	V	strike slip	0.1	15	196
65.01	San Jacinto;SJV+A+CC	CA	n/a	90	V	strike slip	0	16	136

66.04	Elsinore;GI	CA	5	90	V	strike slip	0	13	37
66.04	Elsinore;W+GI	CA	n/a	81	NE	strike slip	0	14	83
67.01	Elsinore;CM	CA	3	82	NE	strike slip	0	13	39
67.57	San Jacinto;A+C	CA	n/a	90	V	strike slip	0	17	118
67.57	San Jacinto;SBV+SJV+A+C	CA	n/a	90	V	strike slip	0	17	181
67.57	San Jacinto;SBV+SJV+A	CA	n/a	90	V	strike slip	0	16	134
67.57	San Jacinto;SJV+A+C	CA	n/a	90	V	strike slip	0	17	136
67.57	San Jacinto;A	CA	9	90	V	strike slip	0	17	71
67.57	San Jacinto;SJV+A	CA	n/a	90	V	strike slip	0	17	89
68.60	San Jacinto;C	CA	14	90	V	strike slip	0	17	47
78.00	San Jacinto;SJV	CA	18	90	V	strike slip	0	16	43
78.00	San Jacinto;SBV+SJV	CA	n/a	90	V	strike slip	0	16	88
80.10	San Jacinto;B	CA	4	90	V	strike slip	0.7	13	34
80.10	San Jacinto;B+SM	CA	n/a	90	V	strike slip	0.4	12	61
83.24	San Joaquin Hills	CA	0.5	23	SW	thrust	2	13	27
84.72	Palos Verdes	CA	3	90	V	strike slip	0	14	99
97.30	Chino, alt 2	CA	1	65	SW	strike slip	0	14	29
98.89	Elsinore;W	CA	2.5	75	NE	strike slip	0	14	46

Search Results

4 earthquakes.

Only List Earthquakes Shown on Map

Format: Magnitude

Sort: Newest First

6.6	The 1968 Borrego Mountain, C…	
	1968-04-09 02:28:58 (UTC)	10.0 km
6.4	The 1954 San Jacinto Fault, Cal…	
	1954-03-19 09:54:27 (UTC)	6.0 km
6.0	16km WSW of Oasis, CA	
	1937-03-25 16:49:02 (UTC)	6.0 km
6.7	The 1918 San Jacinto, Californi…	
	1918-04-21 22:32:30 (UTC)	10.0 km

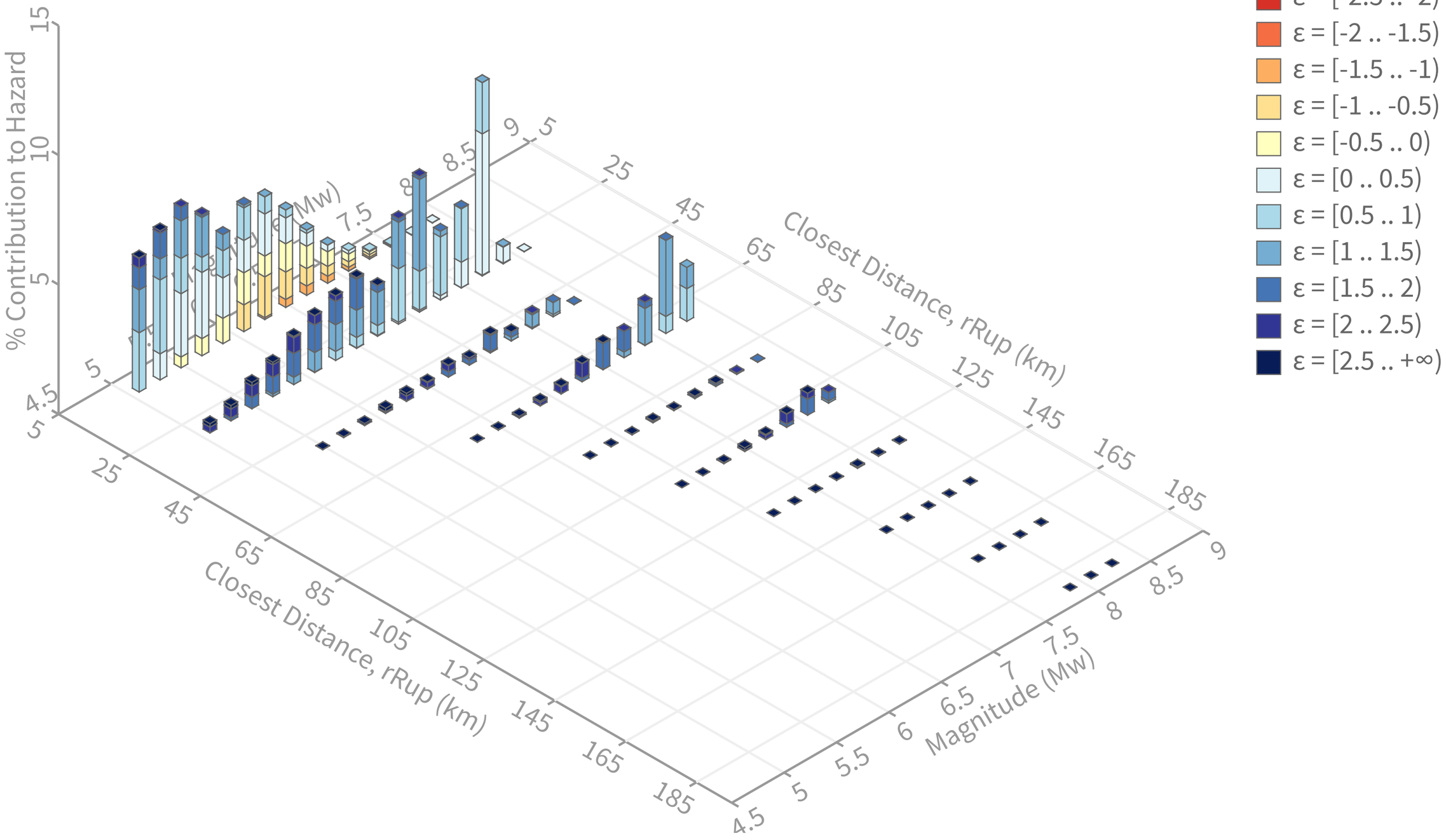
DOWNLOAD

Didn't find what you were looking for?

- Check your Settings.
- Which earthquakes are included on the map and list?
- Felt something not shown – report it here.

Earthquakes Loaded CLOSE

87.658°N : 135.000°E


% Contribution to Hazard
15
10
5
4.5
5
25
45
65
85
105
125
145
165
185
Closest Distance, rRup (km)
5
7.5
8
8.5
9
Magnitude (Mw)
4.5
5
5.5
6
6.5
7
7.5
8
8.5
9
Magnitude (Mw)
5
25
45
65
85
105
125
145
165
185
Closest Distance, rRup (km)
ε = (-∞ .. -2.5)
ε = [-2.5 .. -2)
ε = [-2 .. -1.5)
ε = [-1.5 .. -1)
ε = [-1 .. -0.5)
ε = [-0.5 .. 0)
ε = [0 .. 0.5)
ε = [0.5 .. 1)
ε = [1 .. 1.5)
ε = [1.5 .. 2)
ε = [2 .. 2.5)
ε = [2.5 .. +∞)

Summary statistics for, Deaggregation: Total

Deaggregation targets

Return period: 475 yrs
Exceedance rate: 0.0021052632 yr^{-1}
PGA ground motion: 0.20351854 g

Recovered targets

Return period: 504.90127 yrs
Exceedance rate: 0.0019805852 yr^{-1}

Totals

Binned: 100 %
Residual: 0 %
Trace: 0.22 %

Mean (over all sources)

m: 6.57
r: 27.9 km
ε_0: 0.84 σ

Mode (largest m-r bin)

m: 7.7
r: 30.48 km
ε_0: 0.4 σ
Contribution: 7.46 %

Mode (largest m-r-ε_0 bin)

m: 7.71
r: 30.09 km
ε_0: 0.35 σ
Contribution: 5.46 %

Discretization

r: min = 0.0, max = 1000.0, Δ = 20.0 km
m: min = 4.4, max = 9.4, Δ = 0.2
ε: min = -3.0, max = 3.0, Δ = 0.5 σ

Epsilon keys

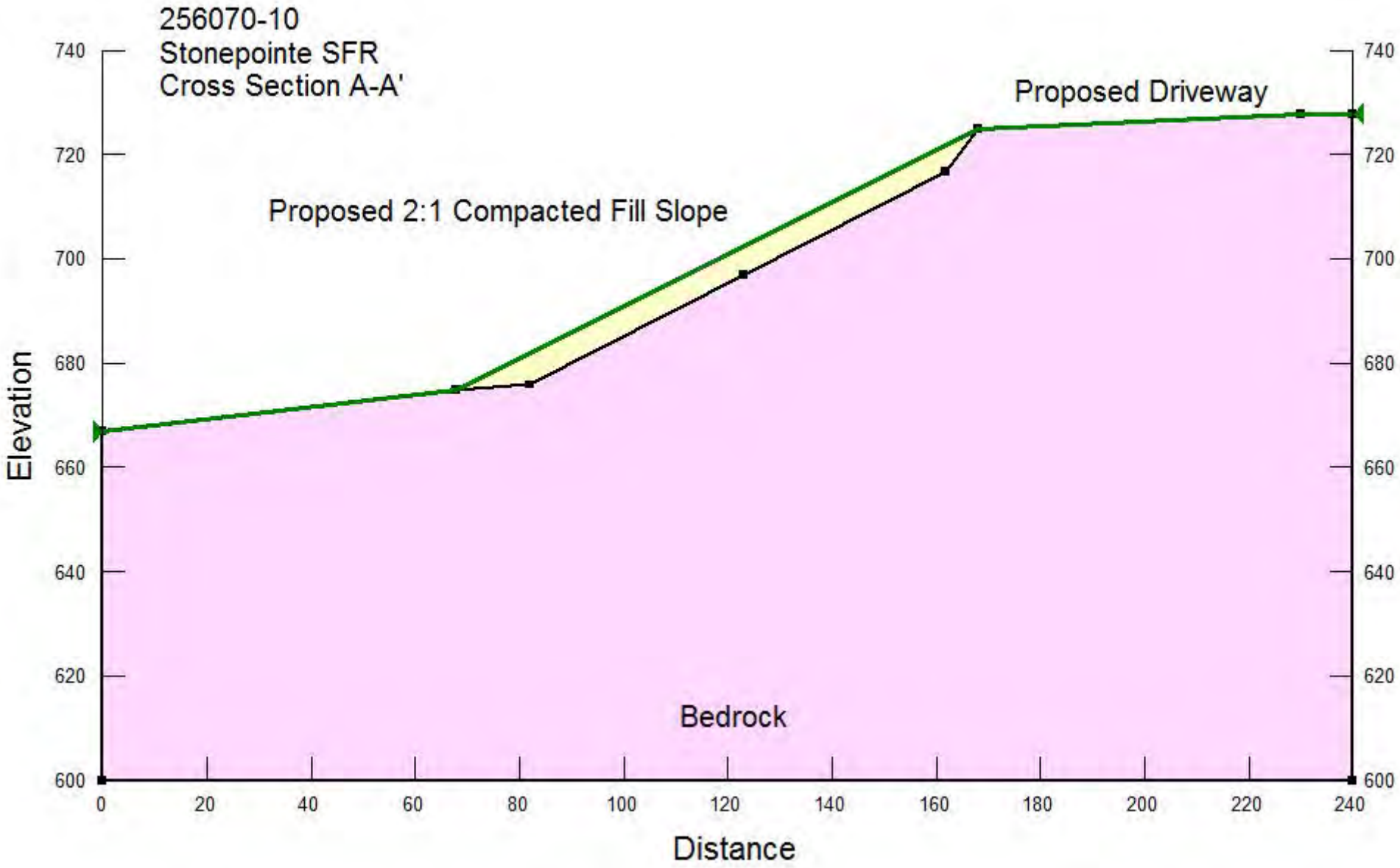
ε0: [-∞ .. -2.5)
ε1: [-2.5 .. -2.0)
ε2: [-2.0 .. -1.5)
ε3: [-1.5 .. -1.0)
ε4: [-1.0 .. -0.5)
ε5: [-0.5 .. 0.0)
ε6: [0.0 .. 0.5)
ε7: [0.5 .. 1.0)
ε8: [1.0 .. 1.5)

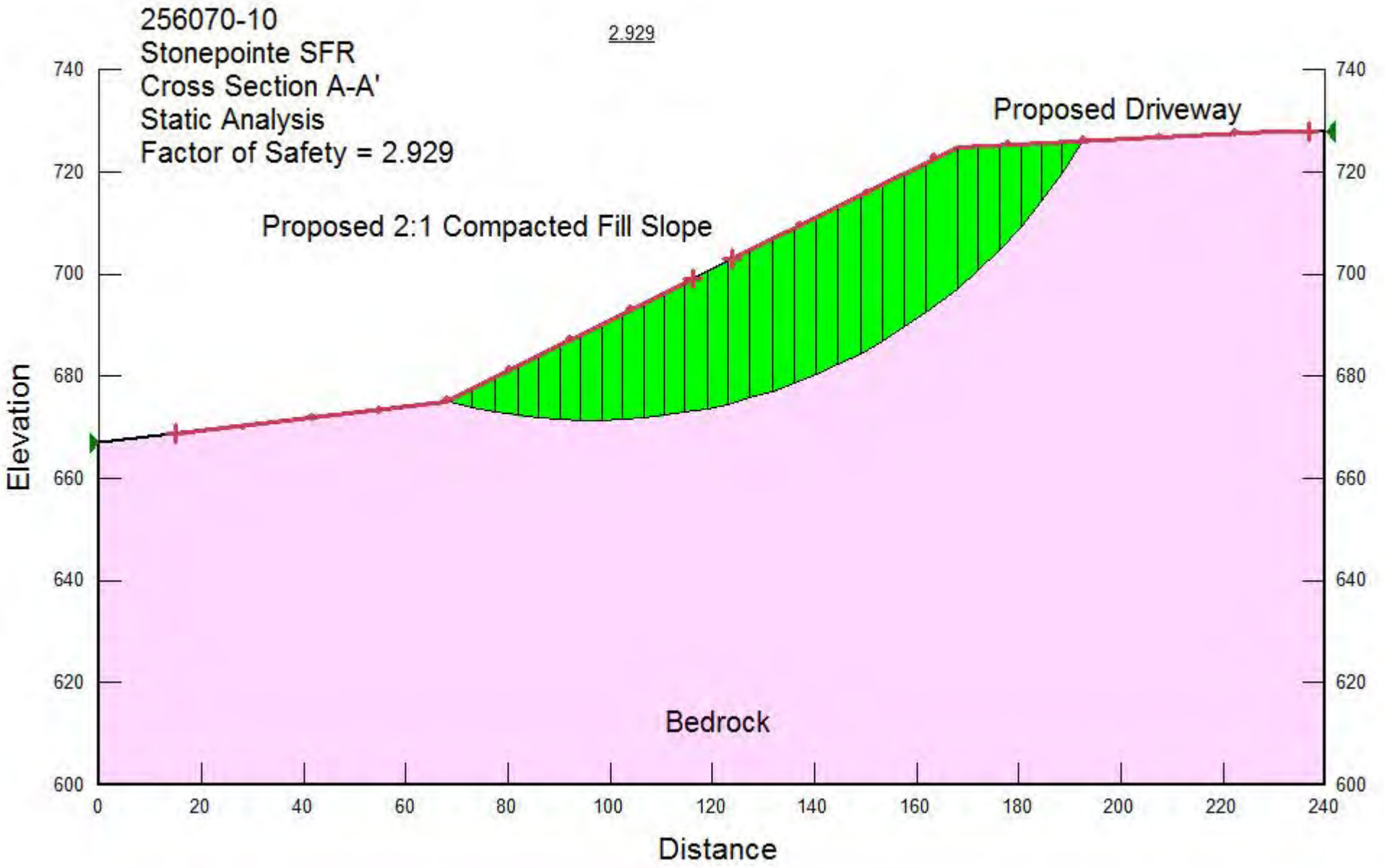
Deaggregation Contributors

Source Set ↳ Source	Type	r	m	ε_0	lon	lat	az	%
UC33brAvg_FM31 (opt)	Grid							28.80
PointSourceFinite: -117.047, 33.121		8.02	5.70	0.07	117.047°W	33.121°N	0.00	3.94
PointSourceFinite: -117.047, 33.121		8.02	5.70	0.07	117.047°W	33.121°N	0.00	3.93
PointSourceFinite: -117.047, 33.139		9.40	5.76	0.23	117.047°W	33.139°N	0.00	2.44
PointSourceFinite: -117.047, 33.139		9.40	5.76	0.23	117.047°W	33.139°N	0.00	2.44
PointSourceFinite: -117.047, 33.202		14.68	5.96	0.74	117.047°W	33.202°N	0.00	1.71
PointSourceFinite: -117.047, 33.202		14.68	5.96	0.74	117.047°W	33.202°N	0.00	1.71
PointSourceFinite: -117.047, 33.193		13.83	5.95	0.65	117.047°W	33.193°N	0.00	1.39
PointSourceFinite: -117.047, 33.193		13.83	5.95	0.65	117.047°W	33.193°N	0.00	1.38
UC33brAvg_FM32 (opt)	Grid							28.72
PointSourceFinite: -117.047, 33.121		8.02	5.70	0.07	117.047°W	33.121°N	0.00	3.93
PointSourceFinite: -117.047, 33.121		8.02	5.70	0.07	117.047°W	33.121°N	0.00	3.93
PointSourceFinite: -117.047, 33.139		9.40	5.76	0.23	117.047°W	33.139°N	0.00	2.44
PointSourceFinite: -117.047, 33.139		9.40	5.76	0.23	117.047°W	33.139°N	0.00	2.43
PointSourceFinite: -117.047, 33.202		14.68	5.96	0.74	117.047°W	33.202°N	0.00	1.71
PointSourceFinite: -117.047, 33.202		14.68	5.96	0.74	117.047°W	33.202°N	0.00	1.70
PointSourceFinite: -117.047, 33.193		13.83	5.95	0.65	117.047°W	33.193°N	0.00	1.39
PointSourceFinite: -117.047, 33.193		13.83	5.95	0.65	117.047°W	33.193°N	0.00	1.38
UC33brAvg_FM31	System							21.62
Elsinore (Julian) [6]		30.56	7.64	0.49	116.877°W	33.297°N	31.11	3.91
Rose Canyon [14]		26.21	7.08	0.78	117.314°W	32.989°N	251.80	3.65
San Jacinto (Coyote Creek) [0]		65.07	8.02	1.22	116.543°W	33.469°N	45.93	2.77
San Jacinto (Anza) rev [4]		67.67	7.93	1.37	116.613°W	33.552°N	36.45	1.81
Earthquake Valley (No Extension) [0]		37.54	7.04	1.30	116.798°W	33.328°N	38.17	1.59
Elsinore (Temecula) rev [5]		31.17	6.97	1.13	117.008°W	33.341°N	6.65	1.14
UC33brAvg_FM32	System							20.86
Elsinore (Julian) [6]		30.56	7.66	0.47	116.877°W	33.297°N	31.11	4.20
Rose Canyon [14]		26.21	7.07	0.79	117.314°W	32.989°N	251.80	3.55
San Jacinto (Coyote Creek) [0]		65.07	8.01	1.23	116.543°W	33.469°N	45.93	2.79
San Jacinto (Anza) rev [4]		67.67	7.93	1.37	116.613°W	33.552°N	36.45	1.77
Earthquake Valley (No Extension) [0]		37.54	7.06	1.28	116.798°W	33.328°N	38.17	1.25

APPENDIX E

SLOPE STABILITY ANALYSIS


256070-10
Stonepointe SFR
Cross Section A-A'
Proposed Driveway
Proposed 2:1 Compacted Fill Slope
Bedrock
Elevation
Distance
740
720
700
680
660
640
620
600
0
20
40
60
80
100
120
140
160
180
200
220
240


256070-10
Stonepointe SFR
Cross Section A-A'
Static Analysis
Factor of Safety = 2.929
2.929
Proposed 2:1 Compacted Fill Slope
Proposed Driveway
Bedrock
Elevation
Distance
740
720
700
680
660
640
620
600
0
20
40
60
80
100
120
140
160
180
200
220
240

SLOPE/W Analysis

Report generated using GeoStudio 2012. Copyright © 1991-2012 GEO-SLOPE Interna onal Ltd.

File Informa on

Created By: Aaron Wood
Last Edited By: Aaron Wood
Revision Number: 2
File Version: 8.0
Tool Version: 8.0.9.6484
Date: 5/27/2025
Time: 3:38:48 PM
File Name: A-A' Sta c.gsz
Directory: C:\Users\awood\Desktop\SlopeW\256070 - Stonepointe\
Last Solved Date: 5/27/2025
Last Solved Time: 3:38:56 PM

Project Se ngs

Length(L) Units: feet
Time(t) Units: Seconds
Force(F) Units: lbf
Pressure(p) Units: psf
Strength Units: psf
Unit Weight of Water: 62.4 pcf
View: 2D

Analysis Se ngs

SLOPE/W Analysis

Kind: SLOPE/W
Method: Spencer
Se ngs
Lambda
Lambda 1: -1
Lambda 2: -0.8
Lambda 3: -0.6
Lambda 4: -0.4
Lambda 5: -0.2
Lambda 6: 0
Lambda 7: 0.2
Lambda 8: 0.4
Lambda 9: 0.6
Lambda 10: 0.8
Lambda 11: 1
PWP Condi ons Source: (none)
Slip Surface
Direc on of movement: Right to Le.
Use Passive Mode: No
Slip Surface Op□on: Entry and Exit

Cri□cal slip surfaces saved: 1
Op□mize Cri□cal Slip Surface Loca□on: No
Tension Crack
Tension Crack Op□on: (none)
F of S Distribu□on
F of S Calcula□on Op□on: Constant
Advanced
Number of Slices: 30
F of S Tolerance: 0.001
Minimum Slip Surface Depth: 0.1 ft
Op□miza□on Maximum Itera□ons: 2,000
Op□miza□on Convergence Tolerance: 1e-007
Star□ng Op□miza□on Points: 8
Ending Op□miza□on Points: 16
Complete Passes per Inser□on: 1
Driving Side Maximum Convex Angle: 5 °
Resis□ng Side Maximum Convex Angle: 1 °

Materials

Bedrock

Model: Mohr-Coulomb
Unit Weight: 150 pcf
Cohesion': 950 psf
Phi': 35 °
Phi-B: 0 °

Compacted Fill

Model: Mohr-Coulomb
Unit Weight: 130.5 pcf
Cohesion': 355 psf
Phi': 38.1 °
Phi-B: 0 °

Slip Surface Entry and Exit

Left Projec□on: Range
Left-Zone Left Coordinate: (15, 668.76471) ft
Left-Zone Right Coordinate: (116.22992, 699.11496) ft
Left-Zone Increment: 8
Right Projec□on: Range
Right-Zone Left Coordinate: (123.85197, 702.92598) ft
Right-Zone Right Coordinate: (237, 728) ft
Right-Zone Increment: 8
Radius Increments: 8

Slip Surface Limits

Left Coordinate: (0, 667) ft
Right Coordinate: (240, 728) ft

Points

	X (ft)	Y (ft)
Point 1	0	667
Point 2	68	675
Point 3	82	676
Point 4	123	697
Point 5	162	717
Point 6	168	725
Point 7	230	728
Point 8	240	728
Point 9	240	600
Point 10	0	600

Regions

	Material	Points	Area (ft²)
Region 1	Bedrock	1,2,3,4,5,6,7,8,9,10	23,454
Region 2	Compacted Fill	2,6,5,4,3	497.5

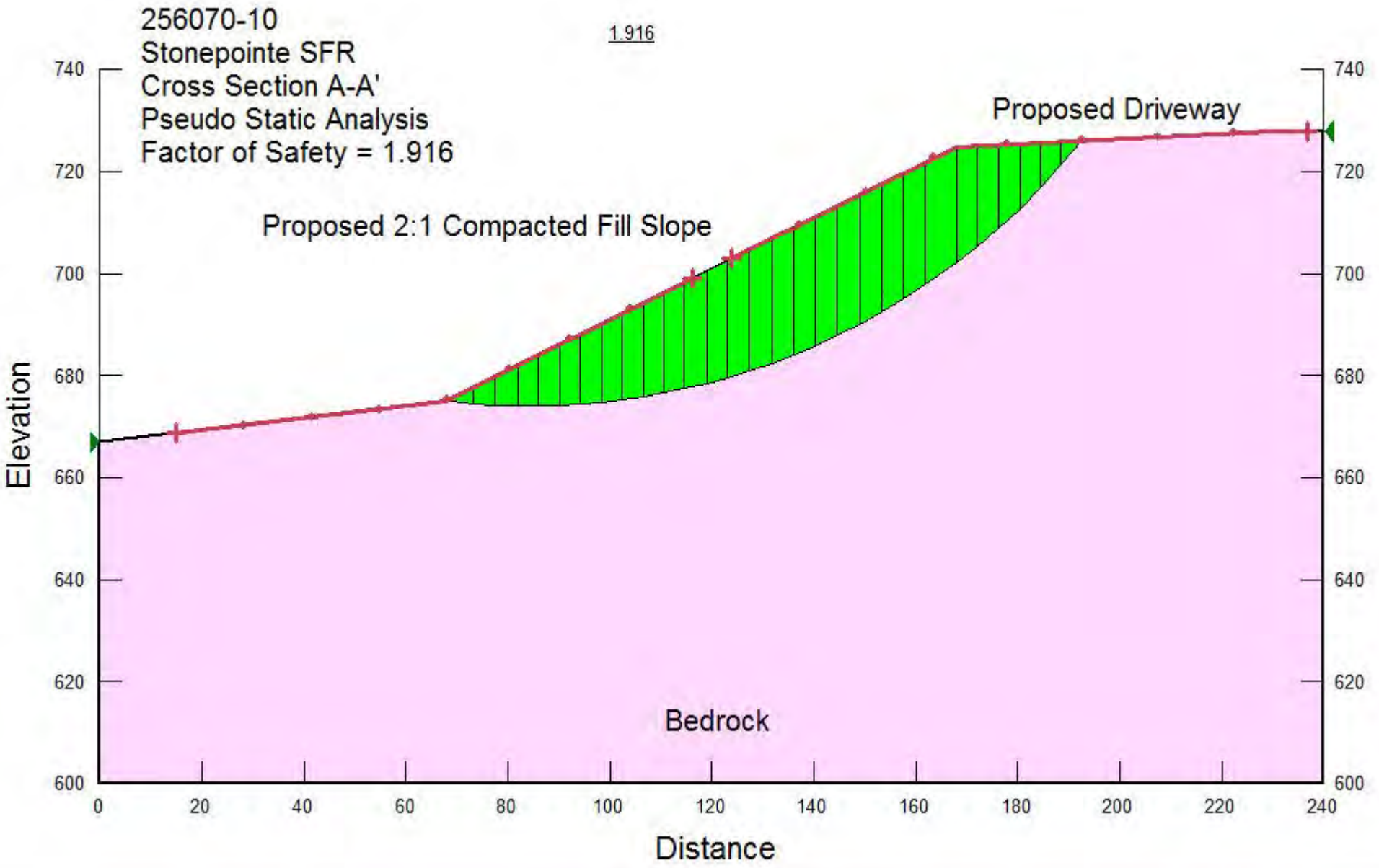
Current Slip Surface

Slip Surface: 374
F of S: 2.929
Volume: 2,616.6229 ft³
Weight: 382,792.78 lbs
Resis□ng Moment: 44,400,740 lbs-ft
Ac□va□ng Moment: 15,160,315 lbs-ft
Resis□ng Force: 354,131.55 lbs
Ac□va□ng Force: 120,953.63 lbs
F of S Rank: 1
Exit: (68.249173, 675.12459) ft
Entry: (192.63189, 726.19187) ft
Radius: 111.54467 ft
Center: (96.635061, 782.99699) ft

Slip Slices

	X (ft)	Y (ft)	PWP (psf)	Base Normal Stress (psf)	Fric□onal Strength (psf)	Cohesive Strength (psf)
Slice 1	68.409525	675.08265	0	115.01597	90.184043	355
Slice 2	70.80823	674.50784	0	563.54271	394.59685	950
Slice 3	75.284938	673.53839	0	1,076.0471	753.45632	950
Slice 4	79.761646	672.75918	0	1,526.1001	1,068.5868	950
Slice 5	84.05	672.18375	0	1,925.7426	1,348.4195	950
Slice 6	88.15	671.79451	0	2,283.2442	1,598.7448	950
Slice 7	92.25	671.55743	0	2,599.294	1,820.0453	950
Slice 8	96.35	671.47152	0	2,876.7276	2,014.3064	950
Slice 9	100.45	671.53645	0	3,117.9058	2,183.1812	950
Slice 10	104.55	671.75247	0	3,324.7971	2,328.048	950

Slice 11	108.65	672.12047	0	3,499.0413	2,450.0551	950
Slice 12	112.75	672.64197	0	3,641.998	2,550.1544	950
Slice 13	116.85	673.31917	0	3,754.7851	2,629.1288	950
Slice 14	120.95	674.15497	0	3,838.3078	2,687.612	950
Slice 15	125.16667	675.18633	0	3,894.1021	2,726.6797	950
Slice 16	129.5	676.42791	0	3,920.9736	2,745.4953	950
Slice 17	133.83333	677.86269	0	3,917.0044	2,742.716	950
Slice 18	138.16667	679.49873	0	3,882.4402	2,718.5139	950
Slice 19	142.5	681.34577	0	3,817.3388	2,672.9294	950
Slice 20	146.83333	683.41557	0	3,721.569	2,605.8707	950
Slice 21	151.16667	685.72239	0	3,594.8041	2,517.1089	950
Slice 22	155.5	688.28361	0	3,436.509	2,406.2695	950
Slice 23	159.83333	691.12057	0	3,245.9206	2,272.8181	950
Slice 24	165	694.94005	0	3,005.4886	2,104.4658	950
Slice 25	170.05266	699.06468	0	2,631.9715	1,842.9263	950
Slice 26	174.15797	702.84553	0	2,151.929	1,506.7969	950
Slice 27	178.26329	707.03669	0	1,652.0223	1,156.7585	950
Slice 28	182.3686	711.71119	0	1,132.1245	792.72211	950
Slice 29	186.47392	716.97291	0	592.72778	415.03246	950
Slice 30	190.57923	722.97891	0	35.820285	25.081634	950


256070-10
Stonepointe SFR
Cross Section A-A'
Pseudo Static Analysis
Factor of Safety = 1.916
1.916
Proposed 2:1 Compacted Fill Slope
Proposed Driveway
Bedrock
Elevation
Distance
740
720
700
680
660
640
620
600
0
20
40
60
80
100
120
140
160
180
200
220
240

SLOPE/W Analysis

Report generated using GeoStudio 2012. Copyright © 1991-2012 GEO-SLOPE Interna onal Ltd.

File Informa on

Created By: Aaron Wood
Last Edited By: Aaron Wood
Revision Number: 5
File Version: 8.0
Tool Version: 8.0.9.6484
Date: 5/27/2025
Time: 3:59:08 PM
File Name: A-A' EQ.gsz
Directory: C:\Users\awood\Desktop\SlopeW\256070 - Stonepointe\
Last Solved Date: 5/27/2025
Last Solved Time: 3:59:13 PM

Project Se ngs

Length(L) Units: feet
Time(t) Units: Seconds
Force(F) Units: lbf
Pressure(p) Units: psf
Strength Units: psf
Unit Weight of Water: 62.4 pcf
View: 2D

Analysis Se ngs

SLOPE/W Analysis

Kind: SLOPE/W
Method: Spencer
Se ngs
Lambda
Lambda 1: -1
Lambda 2: -0.8
Lambda 3: -0.6
Lambda 4: -0.4
Lambda 5: -0.2
Lambda 6: 0
Lambda 7: 0.2
Lambda 8: 0.4
Lambda 9: 0.6
Lambda 10: 0.8
Lambda 11: 1
PWP Condi ons Source: (none)
Slip Surface
Direc on of movement: Right to Le.
Use Passive Mode: No
Slip Surface Op□on: Entry and Exit

Cri□cal slip surfaces saved: 1
Op□mize Cri□cal Slip Surface Loca□on: No
Tension Crack
Tension Crack Op□on: (none)
F of S Distribu□on
F of S Calcula□on Op□on: Constant
Advanced
Number of Slices: 30
F of S Tolerance: 0.001
Minimum Slip Surface Depth: 0.1 ft
Op□miza□on Maximum Itera□ons: 2,000
Op□miza□on Convergence Tolerance: 1e-007
Star□ng Op□miza□on Points: 8
Ending Op□miza□on Points: 16
Complete Passes per Inser□on: 1
Driving Side Maximum Convex Angle: 5 °
Resis□ng Side Maximum Convex Angle: 1 °

Materials

Bedrock

Model: Mohr-Coulomb
Unit Weight: 150 pcf
Cohesion': 950 psf
Phi': 35 °
Phi-B: 0 °

Compacted Fill

Model: Mohr-Coulomb
Unit Weight: 130.5 pcf
Cohesion': 355 psf
Phi': 38.1 °
Phi-B: 0 °

Slip Surface Entry and Exit

Left Projec□on: Range
Left-Zone Left Coordinate: (15, 668.76471) ft
Left-Zone Right Coordinate: (116.22992, 699.11496) ft
Left-Zone Increment: 8
Right Projec□on: Range
Right-Zone Left Coordinate: (123.85197, 702.92598) ft
Right-Zone Right Coordinate: (237, 728) ft
Right-Zone Increment: 8
Radius Increments: 8

Slip Surface Limits

Left Coordinate: (0, 667) ft
Right Coordinate: (240, 728) ft

Seismic Coefficients

Horz Seismic Coef.: 0.21
Ignore seismic load in strength: No

Points

	X (ft)	Y (ft)
Point 1	0	667
Point 2	68	675
Point 3	82	676
Point 4	123	697
Point 5	162	717
Point 6	168	725
Point 7	230	728
Point 8	240	728
Point 9	240	600
Point 10	0	600

Regions

	Material	Points	Area (ft²)
Region 1	Bedrock	1,2,3,4,5,6,7,8,9,10	23,454
Region 2	Compacted Fill	2,6,5,4,3	497.5

Current Slip Surface

Slip Surface: 373
F of S: 1.916
Volume: 2,139.217 ft³
Weight: 311,182.15 lbs
Resis□ng Moment: 45,034,995 lbs-ft
Ac□va□ng Moment: 23,504,552 lbs-ft
Resis□ng Force: 295,274.35 lbs
Ac□va□ng Force: 154,109.59 lbs
F of S Rank: 1
Exit: (68.249173, 675.12459) ft
Entry: (192.63189, 726.19187) ft
Radius: 138.12186 ft
Center: (84.61516, 812.27343) ft

Slip Slices

	X (ft)	Y (ft)	PWP (psf)	Base Normal Stress (psf)	Fric□onal Strength (psf)	Cohesive Strength (psf)
Slice 1	68.532182	675.09141	0	238.99625	187.397	355
Slice 2	71.012659	674.84073	0	790.41204	553.45247	950
Slice 3	75.407595	674.4764	0	1,158.2707	811.02988	950
Slice 4	79.802532	674.25294	0	1,471.3608	1,030.258	950
Slice 5	84.05	674.16793	0	1,748.2906	1,224.1662	950
Slice 6	88.15	674.21203	0	1,995.4783	1,397.2489	950
Slice 7	92.25	674.37802	0	2,206.2802	1,544.8541	950

Slice 8	96.35	674.66634	0	2,383.965	1,669.2703	950
Slice 9	100.45	675.07777	0	2,531.2882	1,772.4271	950
Slice 10	104.55	675.61341	0	2,650.5859	1,855.9603	950
Slice 11	108.65	676.27474	0	2,743.8495	1,921.2641	950
Slice 12	112.75	677.0636	0	2,812.7835	1,969.5322	950
Slice 13	116.85	677.98225	0	2,858.853	2,001.7904	950
Slice 14	120.95	679.03336	0	2,883.3208	2,018.923	950
Slice 15	125.16667	680.25794	0	2,886.8595	2,021.4008	950
Slice 16	129.5	681.6681	0	2,869.2566	2,009.0751	950
Slice 17	133.83333	683.23915	0	2,830.778	1,982.1321	950
Slice 18	138.16667	684.97708	0	2,772.2453	1,941.147	950
Slice 19	142.5	686.88882	0	2,694.362	1,886.6126	950
Slice 20	146.83333	688.98248	0	2,597.7284	1,818.949	950
Slice 21	151.16667	691.26751	0	2,482.8545	1,738.5134	950
Slice 22	155.5	693.75501	0	2,350.1718	1,645.608	950
Slice 23	159.83333	696.45805	0	2,200.0442	1,540.4876	950
Slice 24	165	700.0134	0	2,024.2929	1,417.4252	950
Slice 25	170.05266	703.778	0	1,754.2287	1,228.3242	950
Slice 26	174.15797	707.14273	0	1,407.5388	985.56926	950
Slice 27	178.26329	710.78512	0	1,058.6549	741.27814	950
Slice 28	182.3686	714.73642	0	708.23832	495.91381	950
Slice 29	186.47392	719.03609	0	357.16697	250.09101	950
Slice 30	190.57923	723.73521	0	6.5774632	4.6055893	950

SURFICIAL STABILITY

Earth - Strata, Inc.
Geotechnical, Environmental and Materials Testing Consultants
BETTER PEOPLE • BETTER SERVICE • BETTER RESULTS

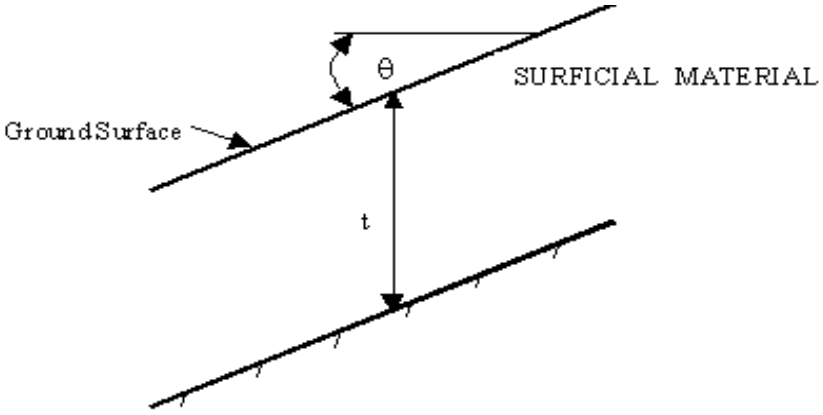
JN: **256070-10** CONSULT: **AGW**
PROJECT: **Stonepointe SFR**

CALCULATION SHEET # **1**

CALCULATE THE SURFICIAL STABILITY OF THE EARTH MATERIAL USING THE INFINITE SLOPE ANALYSIS WITH PARALLEL SEEPAGE. THIS METHOD WAS RECOMMENDED BY THE ASCE AND THE BUILDING AND SAFETY ADVISORY COMMITTEE (8/16/78). MODIFIED FROM SKEMPTON & DeLORY, 1957.

CALCULATION PARAMETERS

EARTH MATERIAL:	Compacted Fill		
COHESION:	355 psf	SHEAR DIAGRAM:	1
PHI ANGLE:	38.1 degrees	SLOPE ANGLE:	27 degrees
DENSITY:	130 pcf	SATURATION DEPTH (t):	4.0 feet

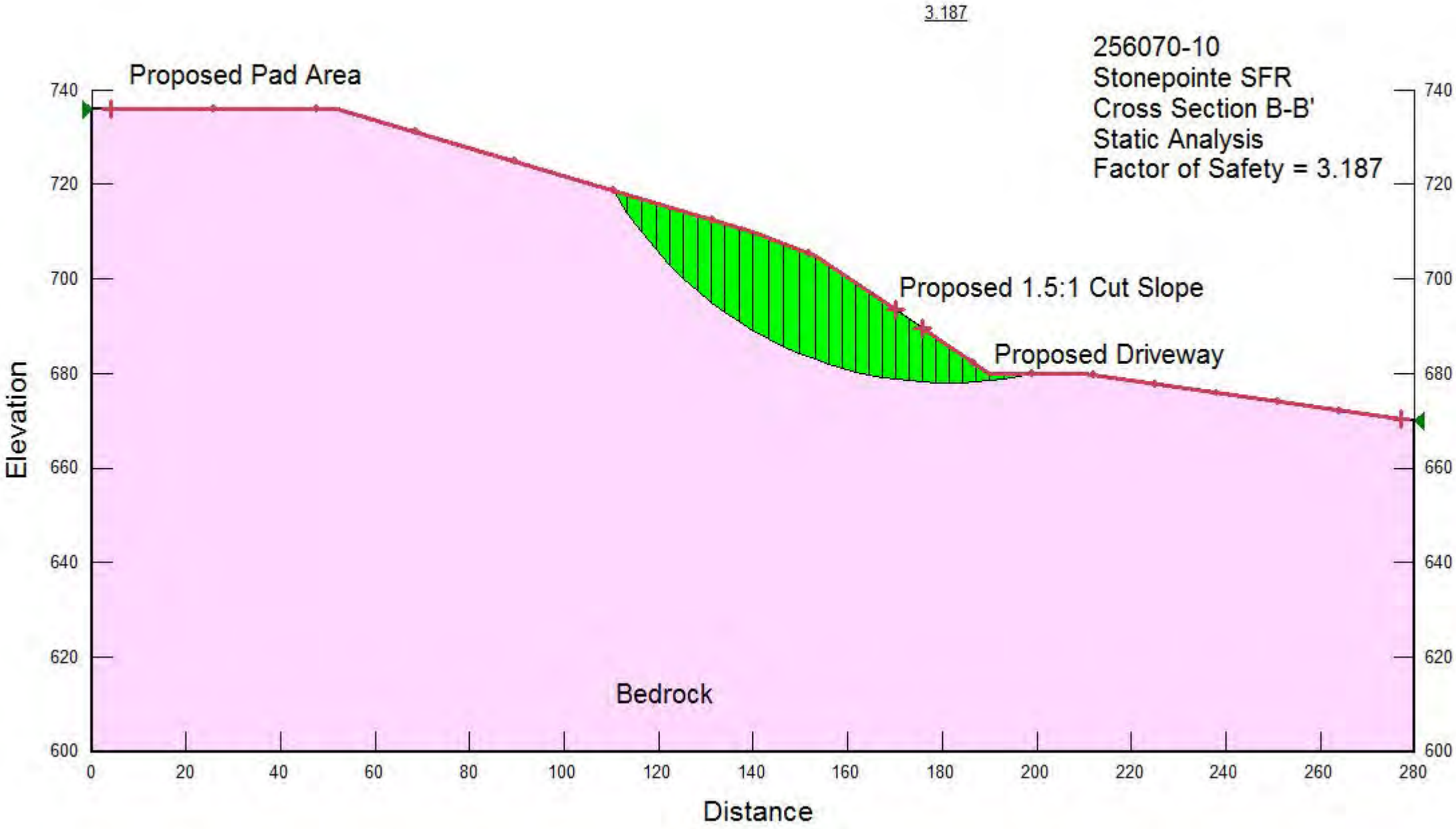


$$FS = \frac{C + (\gamma_{soil} - \gamma_{water}) \bullet t \bullet \cos^2\theta \tan\Phi}{\gamma_{soil} \bullet t \bullet \cos\Phi \sin\Phi}$$

SAFETY FACTOR = 2.49

CONCLUSIONS:

THE CALCULATION INDICATES THAT COMPACTED FILL SLOPES ARE SURFICIALLY STABLE.


3.187
256070-10
Stonepointe SFR
Cross Section B-B'
Static Analysis
Factor of Safety = 3.187
Proposed Pad Area
Proposed 1.5:1 Cut Slope
Proposed Driveway
Bedrock
Elevation
Distance
740
720
700
680
660
640
620
600
0
20
40
60
80
100
120
140
160
180
200
220
240
260
280

SLOPE/W Analysis

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File Informa on

Created By: Aaron Wood
Last Edited By: Aaron Wood
Revision Number: 8
File Version: 8.0
Tool Version: 8.0.9.6484
Date: 5/27/2025
Time: 4:47:17 PM
File Name: B-B' Sta c.gsz
Directory: C:\Users\awood\Desktop\SlopeW\256070 - Stonepointe\
Last Solved Date: 5/27/2025
Last Solved Time: 4:47:21 PM

Project Se ngs

Length(L) Units: feet
Time(t) Units: Seconds
Force(F) Units: lbf
Pressure(p) Units: psf
Strength Units: psf
Unit Weight of Water: 62.4 pcf
View: 2D

Analysis Se ngs

SLOPE/W Analysis

Kind: SLOPE/W
Method: Spencer
Se ngs
Lambda
Lambda 1: -1
Lambda 2: -0.8
Lambda 3: -0.6
Lambda 4: -0.4
Lambda 5: -0.2
Lambda 6: 0
Lambda 7: 0.2
Lambda 8: 0.4
Lambda 9: 0.6
Lambda 10: 0.8
Lambda 11: 1
PWP Condi ons Source: (none)
Slip Surface
Direc on of movement: Le. to Right
Use Passive Mode: No
Slip Surface Op□on: Entry and Exit

Cri□cal slip surfaces saved: 1
Op□mize Cri□cal Slip Surface Loca□on: No
Tension Crack
Tension Crack Op□on: (none)
F of S Distribu□on
F of S Calcula□on Op□on: Constant
Advanced
Number of Slices: 30
F of S Tolerance: 0.001
Minimum Slip Surface Depth: 0.1 ft
Op□miza□on Maximum Itera□ons: 2,000
Op□miza□on Convergence Tolerance: 1e-007
Star□ng Op□miza□on Points: 8
Ending Op□miza□on Points: 16
Complete Passes per Inser□on: 1
Driving Side Maximum Convex Angle: 5 °
Resis□ng Side Maximum Convex Angle: 1 °

Materials

Bedrock

Model: Mohr-Coulomb
Unit Weight: 150 pcf
Cohesion': 950 psf
Phi': 35 °
Phi-B: 0 °

Slip Surface Entry and Exit

Left Projec□on: Range
Left-Zone Left Coordinate: (4.09449, 736) ft
Left-Zone Right Coordinate: (170, 693.51351) ft
Left-Zone Increment: 8
Right Projec□on: Range
Right-Zone Left Coordinate: (175.7925, 689.59966) ft
Right-Zone Right Coordinate: (277, 670.42857) ft
Right-Zone Increment: 8
Radius Increments: 8

Slip Surface Limits

Left Coordinate: (0, 736) ft
Right Coordinate: (280, 670) ft

Points

	X (ft)	Y (ft)
Point 1	0	736
Point 2	52	736
Point 3	140	710
Point 4	153	705

Point 5	190	680
Point 6	210	680
Point 7	280	670
Point 8	280	600
Point 9	0	600

Regions

	Material	Points	Area (ft²)
Region 1	Bedrock	1,2,3,4,5,6,7,8,9	29,566

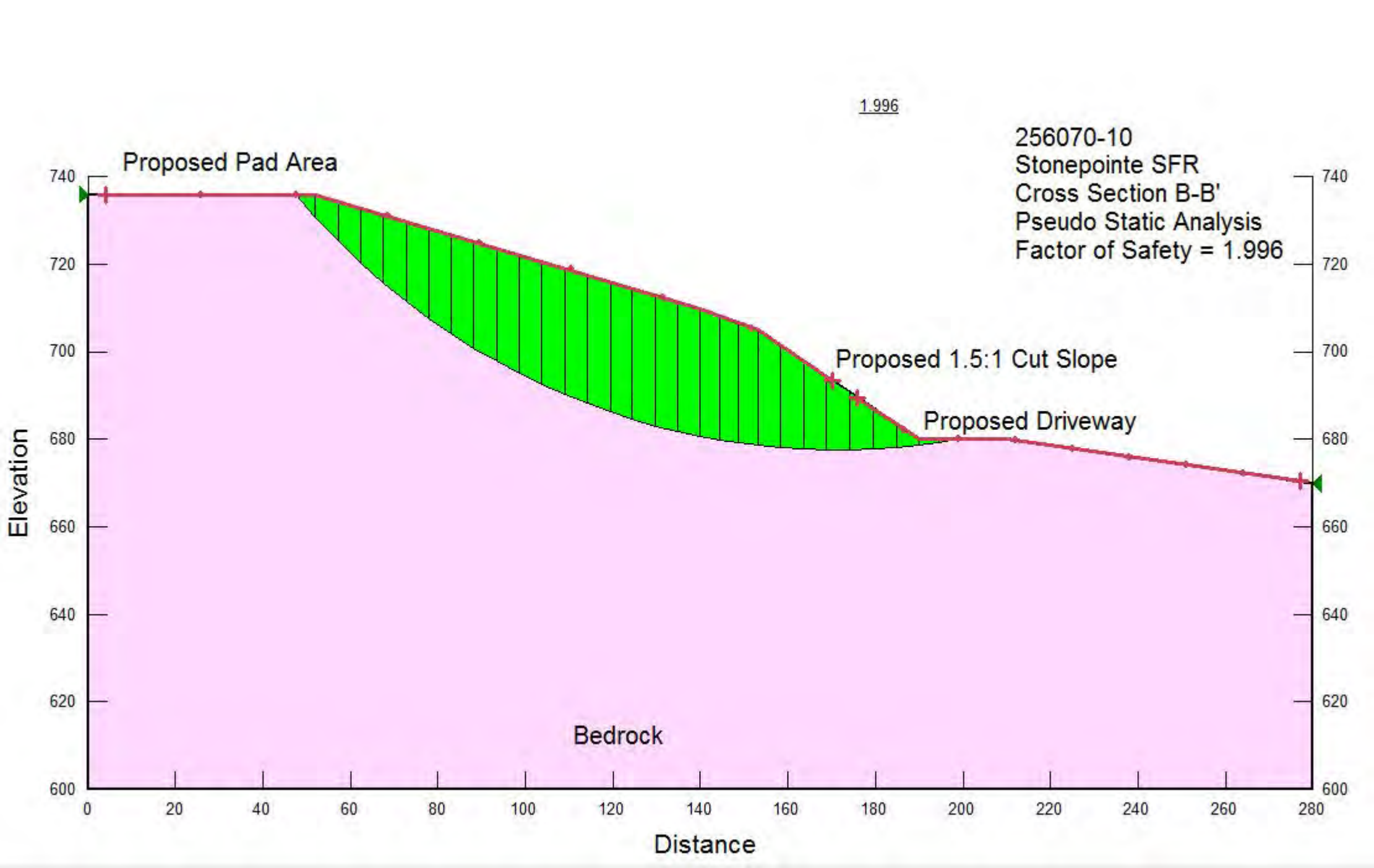
Current Slip Surface

Slip Surface: 428
F of S: 3.187
Volume: 1,160.337 ft³
Weight: 174,050.55 lbs
Resis□ng Moment: 17,556,928 lbs-ft
Ac□va□ng Moment: 5,509,693.2 lbs-ft
Resis□ng Force: 188,869.82 lbs
Ac□va□ng Force: 59,288.233 lbs
F of S Rank: 1
Exit: (199.06009, 680) ft
Entry: (110.41763, 718.74024) ft
Radius: 81.83783 ft
Center: (181.17516, 759.85962) ft

Slip Slices

	X (ft)	Y (ft)	PWP (psf)	Base Normal Stress (psf)	Fric□onal Strength (psf)	Cohesive Strength (psf)
Slice 1	111.89675	716.38331	0	-51.064259	-35.755579	950
Slice 2	114.85499	711.97768	0	298.3769	208.92575	950
Slice 3	117.81323	708.11769	0	626.24824	438.50374	950
Slice 4	120.77146	704.68704	0	932.69765	653.08192	950
Slice 5	123.7297	701.60901	0	1,218.3681	853.1105	950
Slice 6	126.68794	698.82978	0	1,484.0329	1,039.131	950
Slice 7	129.64617	696.3099	0	1,730.4532	1,211.6763	950
Slice 8	132.60441	694.01941	0	1,958.3217	1,371.2316	950
Slice 9	135.56265	691.935	0	2,168.2443	1,518.221	950
Slice 10	138.52088	690.03814	0	2,360.7354	1,653.0048	950
Slice 11	141.625	688.23721	0	2,526.1917	1,768.8585	950
Slice 12	144.875	686.5354	0	2,662.1442	1,864.0534	950
Slice 13	148.125	685.01337	0	2,776.5392	1,944.1537	950
Slice 14	151.375	683.6603	0	2,869.6244	2,009.3327	950
Slice 15	154.42308	682.53247	0	2,882.6495	2,018.4529	950

Slice 16	157.26923	681.6054	0	2,814.7096	1,970.8809	950
Slice 17	160.11538	680.79163	0	2,725.9694	1,908.7443	950
Slice 18	162.96154	680.08768	0	2,616.2058	1,831.887	950
Slice 19	165.80769	679.49065	0	2,485.0809	1,740.0724	950
Slice 20	168.65385	678.99818	0	2,332.136	1,632.9792	950
Slice 21	171.5	678.60836	0	2,156.7824	1,510.1953	950
Slice 22	174.34615	678.31972	0	1,958.2889	1,371.2087	950
Slice 23	177.19231	678.13119	0	1,735.7655	1,215.3961	950
Slice 24	180.03846	678.04207	0	1,488.1412	1,042.0077	950
Slice 25	182.88462	678.05203	0	1,214.1372	850.14804	950
Slice 26	185.73077	678.16112	0	912.23157	638.75142	950
Slice 27	188.57692	678.36973	0	580.61394	406.55026	950
Slice 28	191.51002	678.69126	0	382.98754	268.17077	950
Slice 29	194.53005	679.13333	0	326.97413	228.94975	950
Slice 30	197.55008	679.69157	0	247.26994	173.14027	950


1.996
256070-10
Stonepointe SFR
Cross Section B-B'
Pseudo Static Analysis
Factor of Safety = 1.996
Proposed Pad Area
Proposed 1.5:1 Cut Slope
Proposed Driveway
Bedrock
Elevation
Distance
740
720
700
680
660
640
620
600
0
20
40
60
80
100
120
140
160
180
200
220
240
260
280

SLOPE/W Analysis

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File Informa on

Created By: Aaron Wood
Last Edited By: Aaron Wood
Revision Number: 9
File Version: 8.0
Tool Version: 8.0.9.6484
Date: 5/27/2025
Time: 4:25:54 PM
File Name: B-B' EQ.gsz
Directory: C:\Users\awood\Desktop\SlopeW\256070 - Stonepointe\
Last Solved Date: 5/27/2025
Last Solved Time: 4:25:58 PM

Project Se ngs

Length(L) Units: feet
Time(t) Units: Seconds
Force(F) Units: lbf
Pressure(p) Units: psf
Strength Units: psf
Unit Weight of Water: 62.4 pcf
View: 2D

Analysis Se ngs

SLOPE/W Analysis

Kind: SLOPE/W
Method: Spencer
Se ngs
Lambda
Lambda 1: -1
Lambda 2: -0.8
Lambda 3: -0.6
Lambda 4: -0.4
Lambda 5: -0.2
Lambda 6: 0
Lambda 7: 0.2
Lambda 8: 0.4
Lambda 9: 0.6
Lambda 10: 0.8
Lambda 11: 1
PWP Condi ons Source: (none)
Slip Surface
Direc on of movement: Le. to Right
Use Passive Mode: No
Slip Surface Op□on: Entry and Exit

Cri□cal slip surfaces saved: 1
Op□mize Cri□cal Slip Surface Loca□on: No
Tension Crack
Tension Crack Op□on: (none)
F of S Distribu□on
F of S Calcula□on Op□on: Constant
Advanced
Number of Slices: 30
F of S Tolerance: 0.001
Minimum Slip Surface Depth: 0.1 ft
Op□miza□on Maximum Itera□ons: 2,000
Op□miza□on Convergence Tolerance: 1e-007
Star□ng Op□miza□on Points: 8
Ending Op□miza□on Points: 16
Complete Passes per Inser□on: 1
Driving Side Maximum Convex Angle: 5 °
Resis□ng Side Maximum Convex Angle: 1 °

Materials

Bedrock

Model: Mohr-Coulomb
Unit Weight: 150 pcf
Cohesion': 950 psf
Phi': 35 °
Phi-B: 0 °

Slip Surface Entry and Exit

Left Projec□on: Range
Left-Zone Left Coordinate: (4.09449, 736) ft
Left-Zone Right Coordinate: (170, 693.51351) ft
Left-Zone Increment: 8
Right Projec□on: Range
Right-Zone Left Coordinate: (175.7925, 689.59966) ft
Right-Zone Right Coordinate: (277, 670.42857) ft
Right-Zone Increment: 8
Radius Increments: 8

Slip Surface Limits

Left Coordinate: (0, 736) ft
Right Coordinate: (280, 670) ft

Seismic Coefficients

Horz Seismic Coef.: 0.21
Ignore seismic load in strength: No

Points

	X (ft)	Y (ft)
Point 1	0	736
Point 2	52	736
Point 3	140	710
Point 4	153	705
Point 5	190	680
Point 6	210	680
Point 7	280	670
Point 8	280	600
Point 9	0	600

Regions

	Material	Points	Area (ft²)
Region 1	Bedrock	1,2,3,4,5,6,7,8,9	29,566

Current Slip Surface

Slip Surface: 184
F of S: 1.996
Volume: 2,984.6902 ft³
Weight: 447,703.53 lbs
Resis□ng Moment: 71,631,732 lbs-ft
Ac□va□ng Moment: 35,895,864 lbs-ft
Resis□ng Force: 411,149.09 lbs
Ac□va□ng Force: 206,061.86 lbs
F of S Rank: 1
Exit: (199.06009, 680) ft
Entry: (47.622306, 736) ft
Radius: 160.47463 ft
Center: (171.4435, 838.08046) ft

Slip Slices

	X (ft)	Y (ft)	PWP (psf)	Base Normal Stress (psf)	Fric□onal Strength (psf)	Cohesive Strength (psf)
Slice 1	49.811153	733.45535	0	30.49488	21.352745	950
Slice 2	54.588235	728.15919	0	393.54875	275.5658	950
Slice 3	59.764706	722.89818	0	729.5158	510.81246	950
Slice 4	64.941176	718.09135	0	1,054.0865	738.07931	950
Slice 5	70.117647	713.68578	0	1,366.3566	956.7332	950
Slice 6	75.294118	709.64	0	1,665.6188	1,166.2789	950
Slice 7	80.470588	705.92084	0	1,951.3009	1,366.3156	950
Slice 8	85.647059	702.50134	0	2,222.9161	1,556.5026	950
Slice 9	90.823529	699.35928	0	2,480.0252	1,736.5323	950
Slice 10	96	696.47613	0	2,722.2076	1,906.1103	950
Slice 11	101.17647	693.83635	0	2,949.0392	2,064.9395	950

Slice 12	106.35294	691.42676	0	3,160.0738	2,212.7075	950
Slice 13	111.52941	689.23616	0	3,354.828	2,349.0759	950
Slice 14	116.70588	687.255	0	3,532.7673	2,473.6703	950
Slice 15	121.88235	685.4751	0	3,693.2922	2,586.0711	950
Slice 16	127.05882	683.88949	0	3,835.7244	2,685.8031	950
Slice 17	132.23529	682.49222	0	3,959.2905	2,772.3251	950
Slice 18	137.41176	681.27824	0	4,063.1042	2,845.0162	950
Slice 19	142.16667	680.31443	0	4,114.8533	2,881.2513	950
Slice 20	146.5	679.57141	0	4,114.3681	2,880.9116	950
Slice 21	150.83333	678.94983	0	4,094.7128	2,867.1488	950
Slice 22	155.64286	678.40769	0	3,933.9708	2,754.596	950
Slice 23	160.92857	677.97259	0	3,612.4968	2,529.4975	950
Slice 24	166.21429	677.71285	0	3,238.1158	2,267.3531	950
Slice 25	171.5	677.6276	0	2,805.5417	1,964.4615	950
Slice 26	176.78571	677.71657	0	2,308.3695	1,616.3378	950
Slice 27	182.07143	677.98005	0	1,738.7831	1,217.5091	950
Slice 28	187.35714	678.41891	0	1,087.1673	761.24276	950
Slice 29	192.26502	678.97875	0	697.89216	488.66935	950
Slice 30	196.79507	679.63759	0	614.01817	429.94015	950

APPENDIX F

GENERAL EARTHWORK AND GRADING SPECIFICATIONS

EARTH-STRATA

General Earthwork and Grading Specifications

General

Intent: These General Earthwork and Grading Specifications are intended to be the minimum requirements for the grading and earthwork shown on the approved grading plan(s) and/or indicated in the geotechnical report(s). These General Earthwork and Grading Specifications should be considered a part of the recommendations contained in the geotechnical report(s) and if they are in conflict with the geotechnical report(s), the specific recommendations in the geotechnical report shall supersede these more general specifications. Observations made during earthwork operations by the project Geotechnical Consultant may result in new or revised recommendations that may supersede these specifications and/or the recommendations in the geotechnical report(s).

The Geotechnical Consultant of Record: The Owner shall employ a qualified Geotechnical Consultant of Record (Geotechnical Consultant), prior to commencement of grading or construction. The Geotechnical Consultant shall be responsible for reviewing the approved geotechnical report(s) and accepting the adequacy of the preliminary geotechnical findings, conclusions, and recommendations prior to the commencement of the grading or construction.

Prior to commencement of grading or construction, the Owner shall coordinate with the Geotechnical Consultant, and Earthwork Contractor (Contractor) to schedule sufficient personnel for the appropriate level of observation, mapping, and compaction testing.

During earthwork and grading operations, the Geotechnical Consultant shall observe, map, and document the subsurface conditions to confirm assumptions made during the geotechnical design phase of the project. Should the observed conditions differ significantly from the interpretive assumptions made during the design phase, the Geotechnical Consultant shall recommend appropriate changes to accommodate the observed conditions, and notify the reviewing agency where required.

The Geotechnical Consultant shall observe the moisture conditioning and processing of the excavations and fill materials. The Geotechnical Consultant should perform periodic relative density testing of fill materials to verify that the attained level of compaction is being accomplished as specified.

The Earthwork Contractor: The Earthwork Contractor (Contractor) shall be qualified, experienced, and knowledgeable in earthwork logistics, preparation and processing of earth materials to receive compacted fill, moisture-conditioning and processing of fill, and compacting fill. The Contractor shall be provided with the approved grading plans and geotechnical report(s) for his review and acceptance of responsibilities, prior to commencement of grading. The Contractor shall be solely responsible for performing the grading in accordance with the approved grading plans and geotechnical report(s). Prior to commencement of grading, the Contractor shall prepare and submit to the Owner and the Geotechnical Consultant a work plan that indicates the sequence of earthwork grading, the number of "equipment" of work and the estimated quantities of daily earthwork contemplated for the site. The Contractor shall inform the Owner and the Geotechnical Consultant of work schedule changes and revisions to the work plan at least 24 hours in advance of such changes so that appropriate personnel will be available for observation and testing. No assumptions shall be made by the Contractor with regard to whether the Geotechnical Consultant is aware of all grading operations.

It is the sole responsibility of the Contractor to provide adequate equipment and methods to accomplish the earthwork operations in accordance with the applicable grading codes and agency ordinances, these specifications, and the recommendations in the approved geotechnical report(s) and grading plan(s). At the sole discretion of the Geotechnical Consultant, any unsatisfactory conditions, such as unsuitable earth materials, improper moisture conditioning, inadequate compaction, insufficient buttress keyway size, adverse weather conditions, etc., resulting in a quality of work less than required in the approved grading plans and geotechnical report(s), the Geotechnical Consultant shall reject the work and may recommend to the Owner that grading be stopped until conditions are corrected.

Preparation of Areas for Compacted Fill

Clearing and Grubbing: Vegetation, such as brush, grass, roots, and other deleterious material shall be sufficiently removed and properly disposed in a method acceptable to the Owner, Geotechnical Consultant, and governing agencies.

The Geotechnical Consultant shall evaluate the extent of these removals on a site by site basis. Earth materials to be placed as compacted fill shall not contain more than 1 percent organic materials (by volume). No compacted fill lift shall contain more than 10 percent organic matter.

Should potentially hazardous materials be encountered, the Contractor shall stop work in the affected area, and a hazardous materials specialist shall immediately be consulted to evaluate the potentially hazardous materials, prior to continuing to work in that area.

It is our understanding that the State of California defines most refined petroleum products (gasoline, diesel fuel, motor oil, grease, coolant, etc.) as hazardous waste. As such, indiscriminate dumping or spillage of these fluids may constitute a misdemeanor, punishable by fines and/or imprisonment, and shall be prohibited. The contractor is responsible for all hazardous waste related to his operations. The Geotechnical Consultant does not have expertise in this area. If hazardous waste is a concern, then the Owner should contract the services of a qualified environmental assessor.

Processing: Exposed earth materials that have been observed to be satisfactory for support of compacted fill by the Geotechnical Consultant shall be scarified to a minimum depth of 6 inches. Exposed earth materials that are not observed to be satisfactory shall be removed or alternative recommendations may be provided by the Geotechnical Consultant. Scarification shall continue until the exposed earth materials are broken down and free of oversize material and the working surface is reasonably uniform, flat, and free of uneven features that would inhibit uniform compaction. The earth materials should be moistened or air dried to near optimum moisture content, prior to compaction.

Overexcavation: The Cut Lot Typical Detail and Cut/Fill Transition Lot Typical Detail, included herein provides a graphic illustration that depicts typical overexcavation recommendations made in the approved geotechnical report(s) and/or grading plan(s).

Keyways and Benching: Where fills are to be placed on slopes steeper than 5:1 (horizontal to vertical units), the ground shall be thoroughly benched as compacted fill is placed. Please see the three Keyway and Benching Typical Details with subtitles Cut Over Fill Slope, Fill Over Cut Slope, and Fill Slope for a graphic illustration. The lowest bench or smallest keyway shall be a minimum of 15 feet wide (or ½ the proposed slope height) and at least 2 feet into competent earth materials as advised by the Geotechnical Consultant. Typical benches shall be excavated a minimum height of 4 feet into competent earth materials or as recommended by the Geotechnical Consultant. Fill placed on slopes steeper than 5:1 should be thoroughly benched or otherwise excavated to provide a flat subgrade for the compacted fill.

Evaluation/Acceptance of Bottom Excavations: All areas to receive compacted fill (bottom excavations), including removal excavations, processed areas, keyways, and benching, shall be observed, mapped, general elevations recorded, and/or tested prior to being accepted by the Geotechnical Consultant as suitable to receive compacted fill. The Contractor shall obtain a written acceptance from the Geotechnical Consultant prior to placing compacted fill. A licensed surveyor shall provide the survey control for determining elevations of bottom excavations, processed areas, keyways, and

benching. The Geotechnical Consultant is not responsible for erroneously located, fills, subdrain systems, or excavations.

Fill Materials

General: Earth material to be used as compacted fill should to a large extent be free of organic matter and other deleterious substances as evaluated and accepted by the Geotechnical Consultant.

Oversize: Oversize material is rock that does not break down into smaller pieces and has a maximum diameter greater than 8 inches. Oversize rock shall not be included within compacted fill unless specific methods and guidelines acceptable to the Geotechnical Consultant are followed. For examples of methods and guidelines of oversize rock placement see the enclosed Oversize Rock Disposal Detail. The inclusion of oversize materials in the compacted fill shall only be acceptable if the oversize material is completely surrounded by compacted fill or thoroughly jetted granular materials. No oversize material shall be placed within 10 vertical feet of finish grade or within 2 feet of proposed utilities or underground improvements.

Import: Should imported earth materials be required, the proposed import materials shall meet the requirements of the Geotechnical Consultant. Well graded, very low expansion potential earth materials free of organic matter and other deleterious substances are usually sought after as import materials. However, it is generally in the Owners best interest that potential import earth materials are provided to the Geotechnical Consultant to determine their suitability for the intended purpose. At least 48 hours should be allotted for the appropriate laboratory testing to be performed, prior to starting the import operations.

Fill Placement and Compaction Procedures

Fill Layers: Fill materials shall be placed in areas prepared to receive fill in nearly horizontal layers not exceeding 8 inches in loose thickness. Thicker layers may be accepted by the Geotechnical Consultant, provided field density testing indicates that the grading procedures can adequately compact the thicker layers. Each layer of fill shall be spread evenly and thoroughly mixed to obtain uniformity within the earth materials and consistent moisture throughout the fill.

Moisture Conditioning of Fill: Earth materials to be placed as compacted fill shall be watered, dried, blended, and/or mixed, as needed to obtain relatively uniform moisture contents that are at or slightly above optimum. The maximum density and optimum moisture content tests should be performed in accordance with the American Society of Testing and Materials (ASTM test method D1557-00).

Compaction of Fill: After each layer has been moisture-conditioned, mixed, and evenly spread, it should be uniformly compacted to a minimum of 90 percent of maximum dry density as determined by ASTM test method D1557-00. Compaction equipment shall be adequately sized and be either specifically designed for compaction of earth materials or be proven to consistently achieve the required level of compaction.

Compaction of Fill Slopes: In addition to normal compaction procedures specified above, additional effort to obtain compaction on slopes is needed. This may be accomplished by backrolling of slopes with sheepsfoot rollers as the fill is being placed, by overbuilding the fill slopes, or by other methods producing results that are satisfactory to the Geotechnical Consultant. Upon completion of grading, relative compaction of the fill and the slope face shall be a minimum of 90 percent of maximum density per ASTM test method D1557-00.

Compaction Testing of Fill: Field tests for moisture content and relative density of the compacted fill earth materials shall be periodically performed by the Geotechnical Consultant. The location and frequency of tests shall be at the Geotechnical Consultant's discretion based on field observations. Compaction test locations will not necessarily be random. The test locations may or may not be selected to verify minimum compaction requirements in areas that are typically prone to inadequate compaction, such as close to slope faces and near benching.

Frequency of Compaction Testing: Compaction tests shall be taken at minimum intervals of every 2 vertical feet and/or per 1,000 cubic yards of compacted materials placed. Additionally, as a guideline, at least one (1) test shall be taken on slope faces for each 5,000 square feet of slope face and/or for each 10 vertical feet of slope. The Contractor shall assure that fill placement is such that the testing schedule described herein can be accomplished by the Geotechnical Consultant. The Contractor shall stop or slow down the earthwork operations to a safe level so that these minimum standards can be obtained.

Compaction Test Locations: The approximate elevation and horizontal coordinates of each test location shall be documented by the Geotechnical Consultant. The Contractor shall coordinate with the Surveyor to assure that sufficient grade stakes are established. This will provide the Geotechnical Consultant with sufficient accuracy to determine the approximate test locations and elevations. The Geotechnical Consultant can not be responsible for staking erroneously located by the Surveyor or Contractor. A minimum of two grade stakes should be provided at a maximum horizontal distance of 100 feet and vertical difference of less than 5 feet.

Subdrain System Installation

Subdrain systems shall be installed in accordance with the approved geotechnical report(s), the approved grading plan, and the typical details provided herein. The Geotechnical Consultant may recommend additional subdrain systems and/or changes to the subdrain systems described herein, with regard to the extent, location, grade, or material depending on conditions encountered during grading or other factors. All subdrain systems shall be surveyed by a licensed land surveyor (except for retaining wall subdrain systems) to verify line and grade after installation and prior to burial. Adequate time should be allowed by the Contractor to complete these surveys.

Excavation

All excavations and over-excavations for remedial purposes shall be evaluated by the Geotechnical Consultant during grading operations. Remedial removal depths indicated on the geotechnical plans are estimates only. The actual removal depths and extent shall be determined by the Geotechnical Consultant based on the field evaluation of exposed conditions during grading operations. Where fill over cut slopes are planned, the cut portion of the slope shall be excavated, evaluated, and accepted by the Geotechnical Consultant prior to placement of the fill portion of the proposed slope, unless specifically addressed by the Geotechnical Consultant. Typical details for cut over fill slopes and fill over cut slopes are provided herein.

Trench Backfill

1) The Contractor shall follow all OHSA and Cal/OSHA requirements for trench excavation safety.

2) Bedding and backfill of utility trenches shall be done in accordance with the applicable provisions in the Standard Specifications of Public Works Construction. Bedding materials shall have a Sand Equivalency more than 30 (SE>30). The bedding shall be placed to 1 foot over the conduit and thoroughly jetting to provide densification. Backfill should be compacted to a minimum of 90 percent of maximum dry density, from 1 foot above the top of the conduit to the surface.

3) Jetting of the bedding materials around the conduits shall be observed by the Geotechnical Consultant.

4) The Geotechnical Consultant shall test trench backfill for the minimum compaction requirements recommended herein. At least one test should be conducted for every 300 linear feet of trench and for each 2 vertical feet of backfill.

5) For trench backfill the lift thicknesses shall not exceed those allowed in the Standard Specifications of Public Works Construction, unless the Contractor can demonstrate to the Geotechnical Consultant that the fill lift can be compacted to the minimum relative compaction by his alternative equipment or method.

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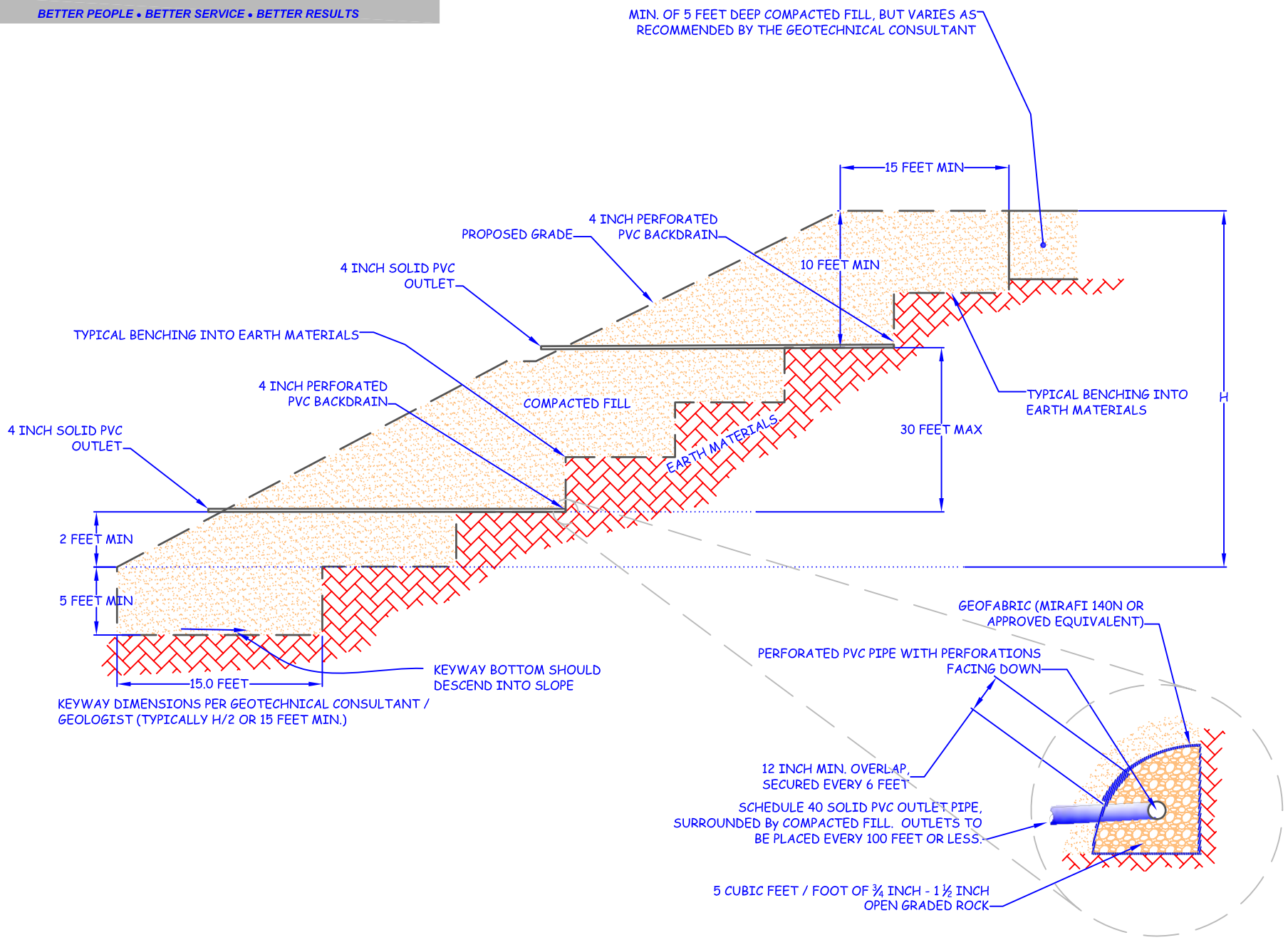
STABILIZATION FILL TYPICAL DETAIL



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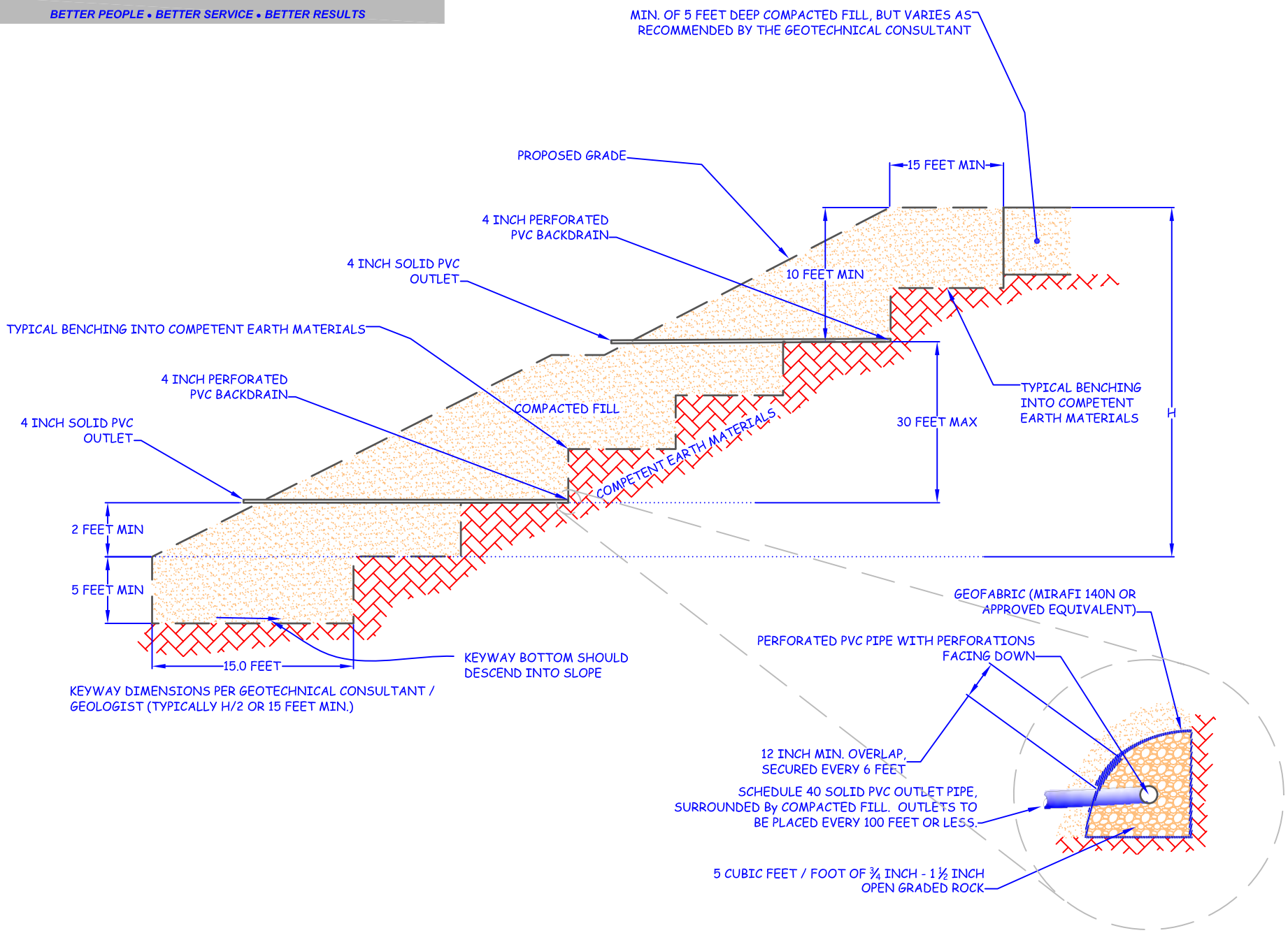
BUTTRESS TYPICAL DETAIL



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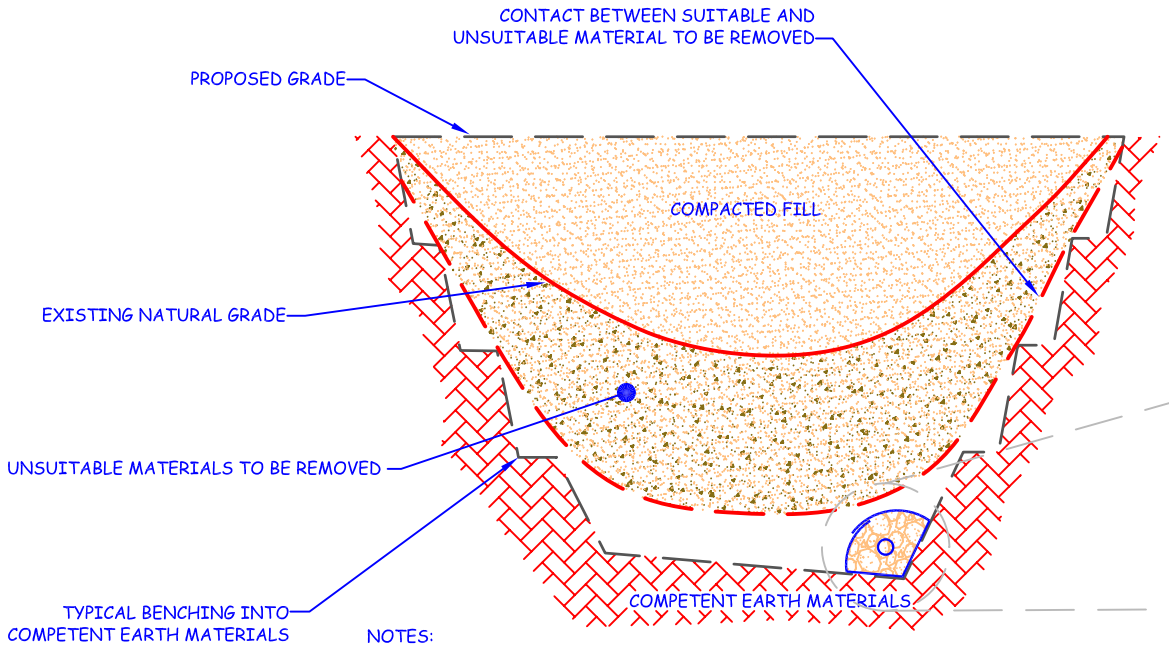
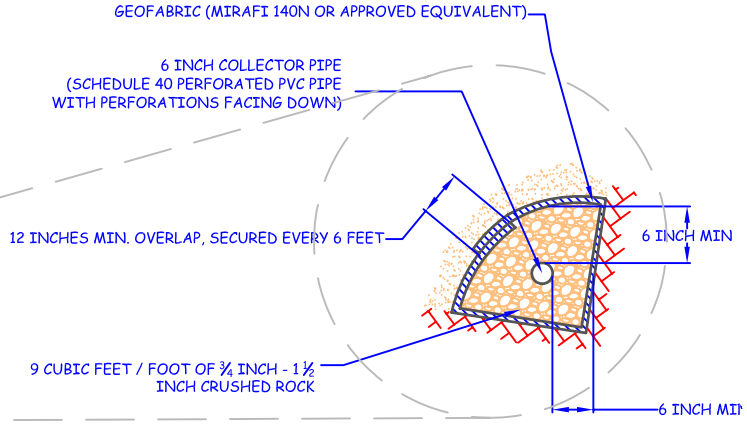
CANYON SUBDRAIN SYSTEM TYPICAL DETAIL

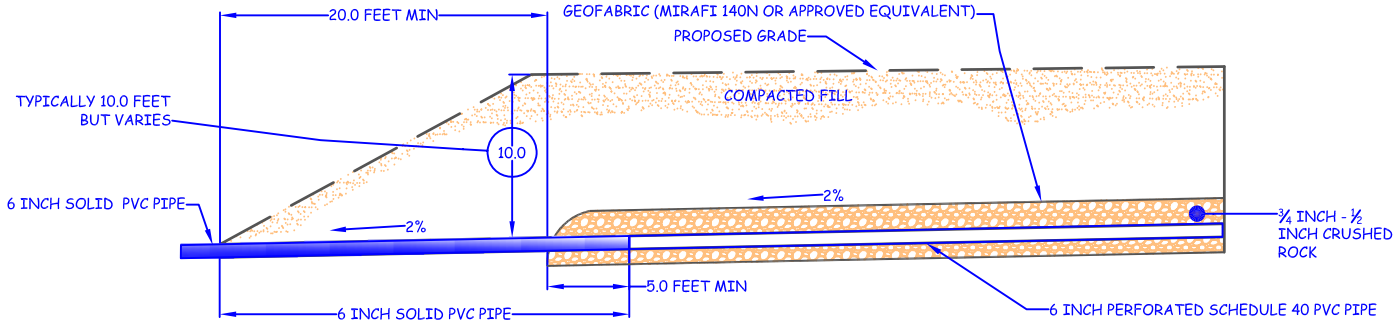




NOTES:
1 - CONTINUOUS RUNS IN EXCESS OF 500 FEET LONG WILL REQUIRE AN 8 INCH DIAMETER PIPE.

2 - FINAL 20 FEET OF PIPE AT OUTLET WILL BE SOLID AND BACKFILLED WITH COMPACTED FINE-GRAINED EARTH MATERIALS.

CANYON SUBDRAIN TYPICAL OUTLET



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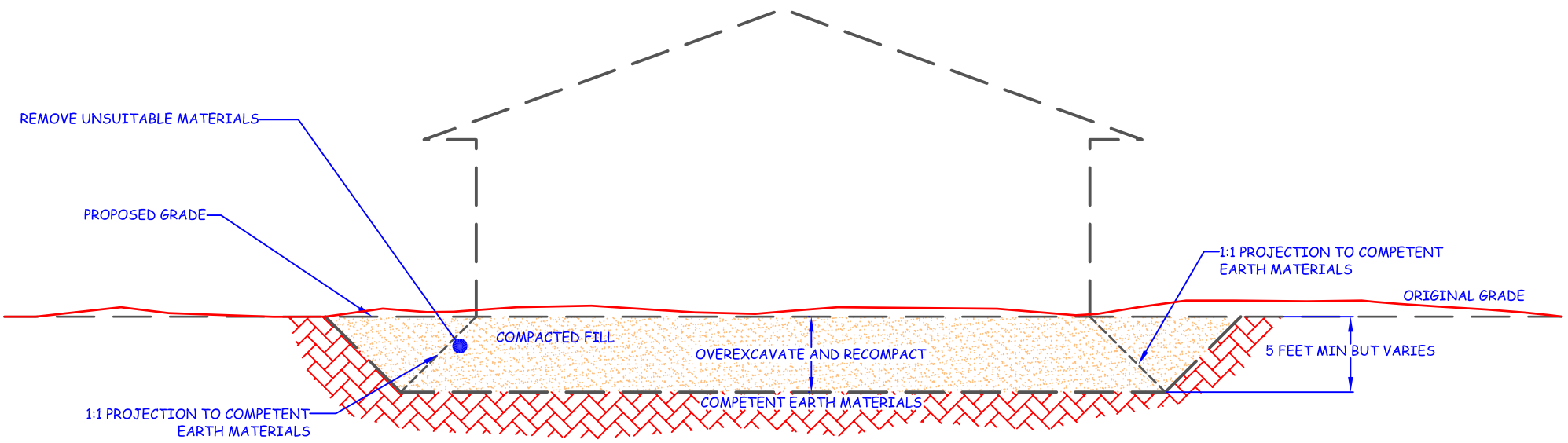
CUT LOT TYPICAL DETAIL



NOTE; REMOVAL BOTTOMS SHOULD BE GRADED WITH A MINIMUM 2% FALL TOWARDS STREET OR OTHER SUITABLE AREA (AS DETERMINED BY THE GEOTECHNICAL CONSULTANT) TO AVOID PONDING BELOW THE BUILDING

NOTE: WHERE DESIGN CUT LOTS ARE EXCAVATED ENTIRELY INTO COMPETENT EARTH MATERIALS, OVEREXCAVATION MAY STILL BY NEEDED FOR HARD-ROCK CONDITIONS OR MATERIALS WITH VARIABLE EXPANSION POTENTIALS

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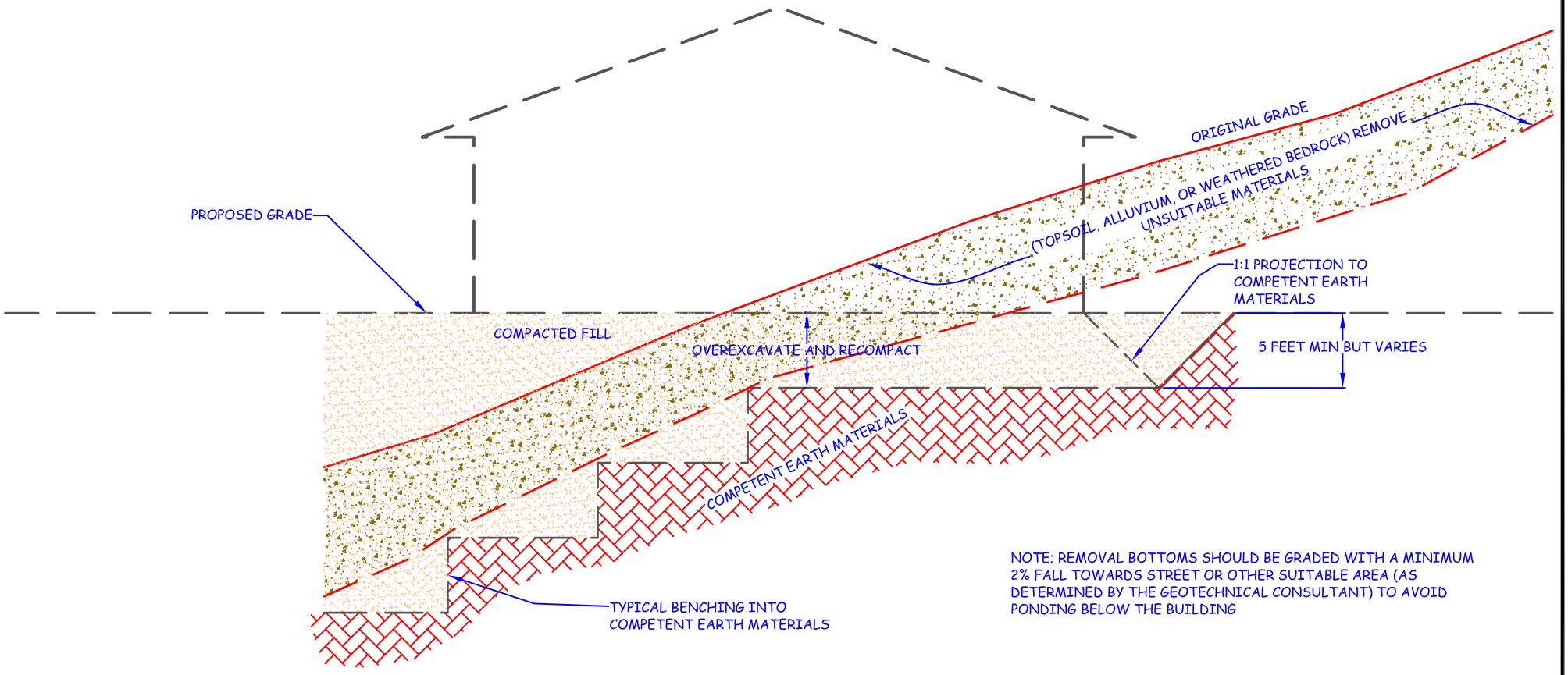
CUT / FILL TRANSITION LOT TYPICAL DETAIL



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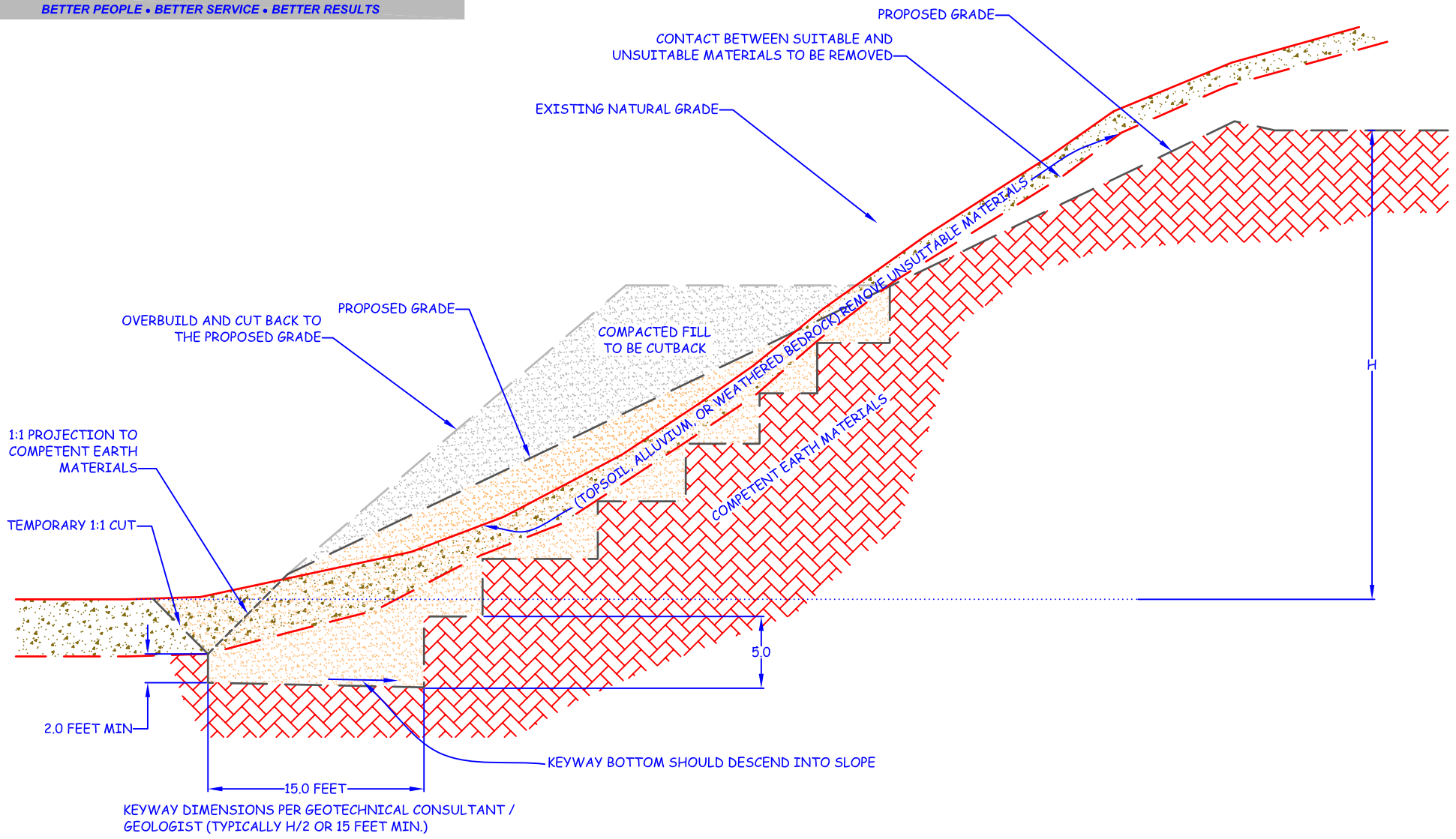
KEYWAY & BENCHING TYPICAL DETAILS
CUT OVER FILL SLOPE



NOTE:

NATURAL SLOPES STEEPER THAN 5:1 (H:V) MUST BE BENCHED INTO COMPETENT EARTH MATERIALS

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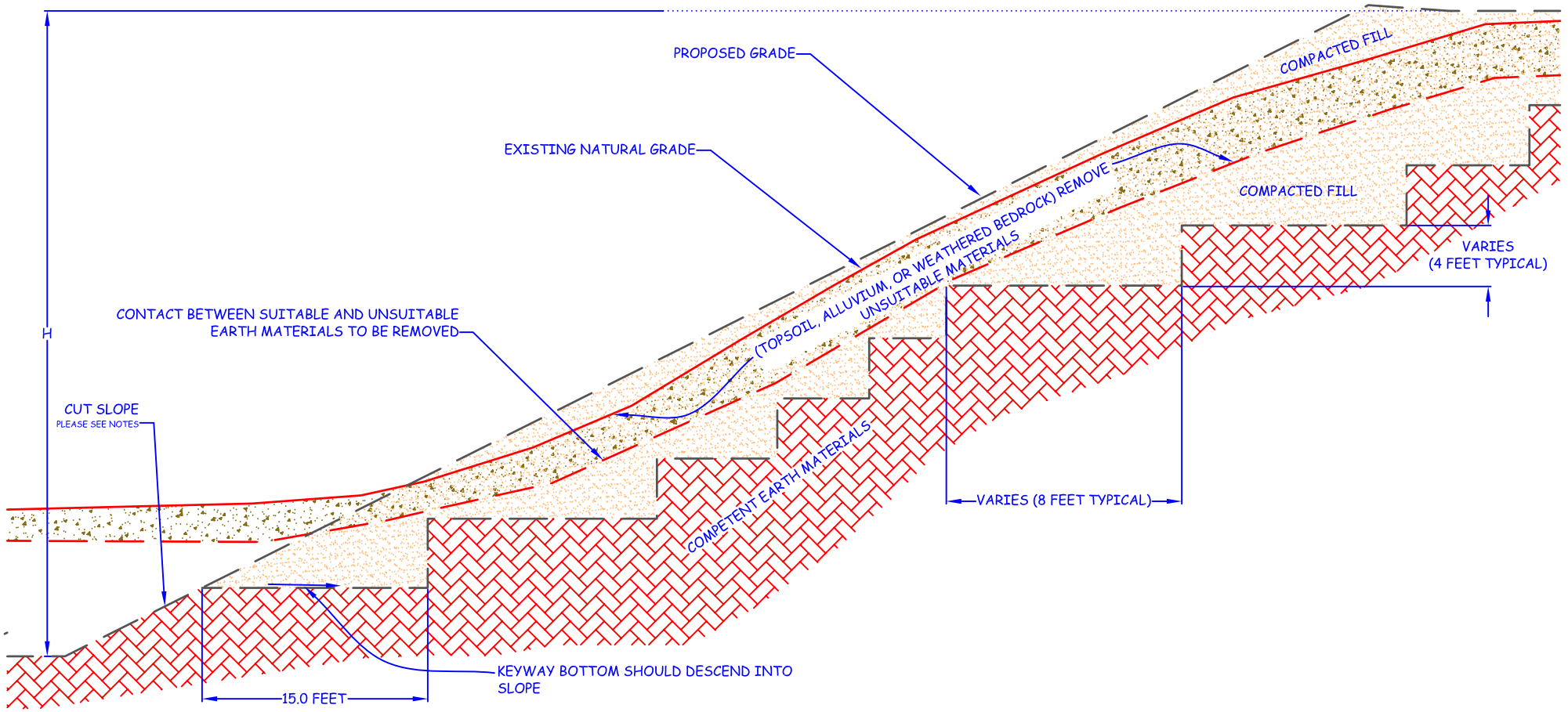
KEYWAY & BENCHING TYPICAL DETAILS
FILL OVER CUT SLOPE



KEYWAY DIMENSIONS PER GEOTECHNICAL CONSULTANT / GEOLOGIST (TYPICALLY H/2 OR 15 FEET MIN.)

NOTES:

NATURAL SLOPES STEEPER THAN 5:1 (H:V) MUST BE BENCHED INTO COMPETENT EARTH MATERIALS

THE CUT SLOPE MUST BE CONSTRUCTED FIRST

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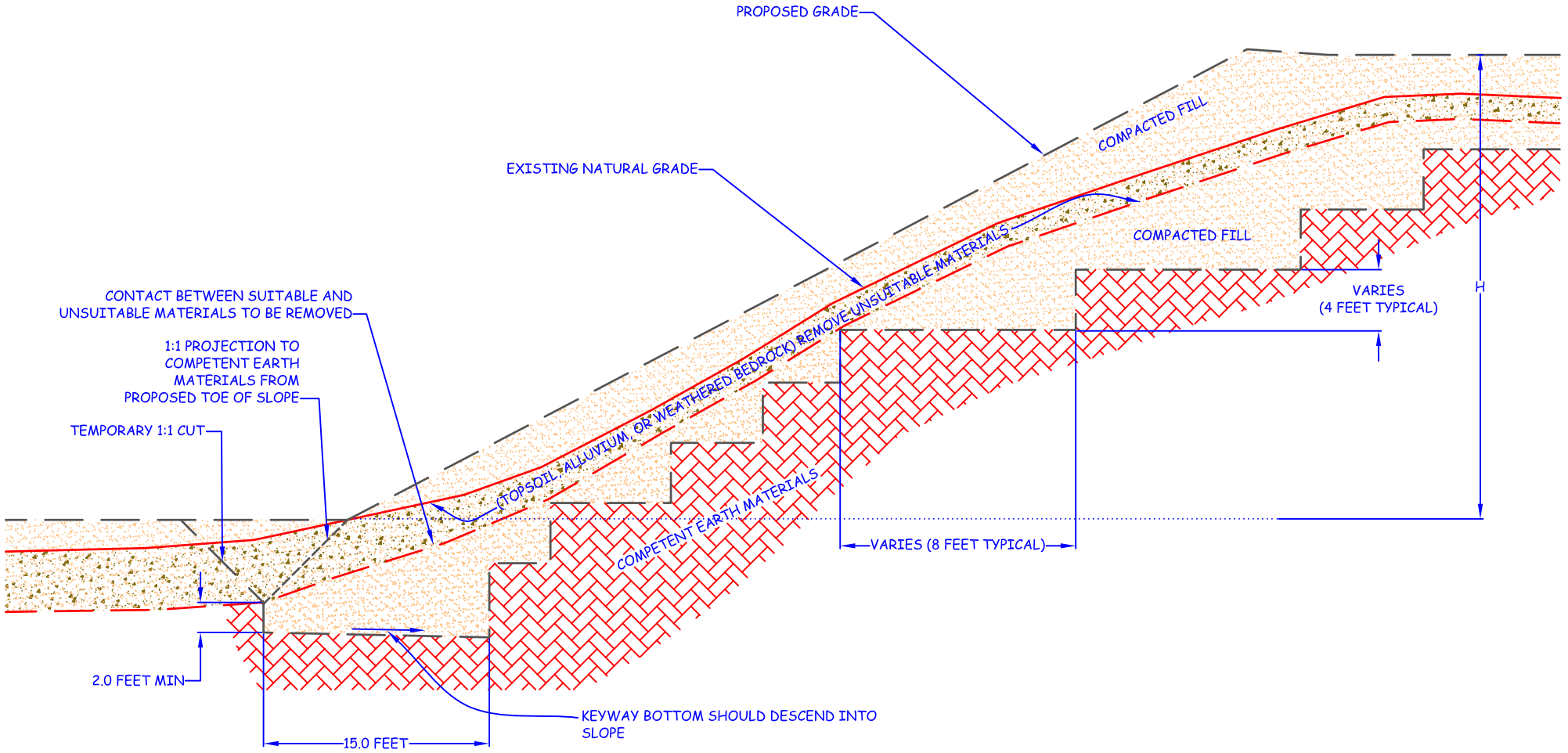
KEYWAY & BENCHING TYPICAL DETAILS
FILL SLOPE



NOTES:

NATURAL SLOPES STEEPER THAN 5:1 (H:V) MUST BE BENCHED INTO COMPETENT EARTH MATERIALS

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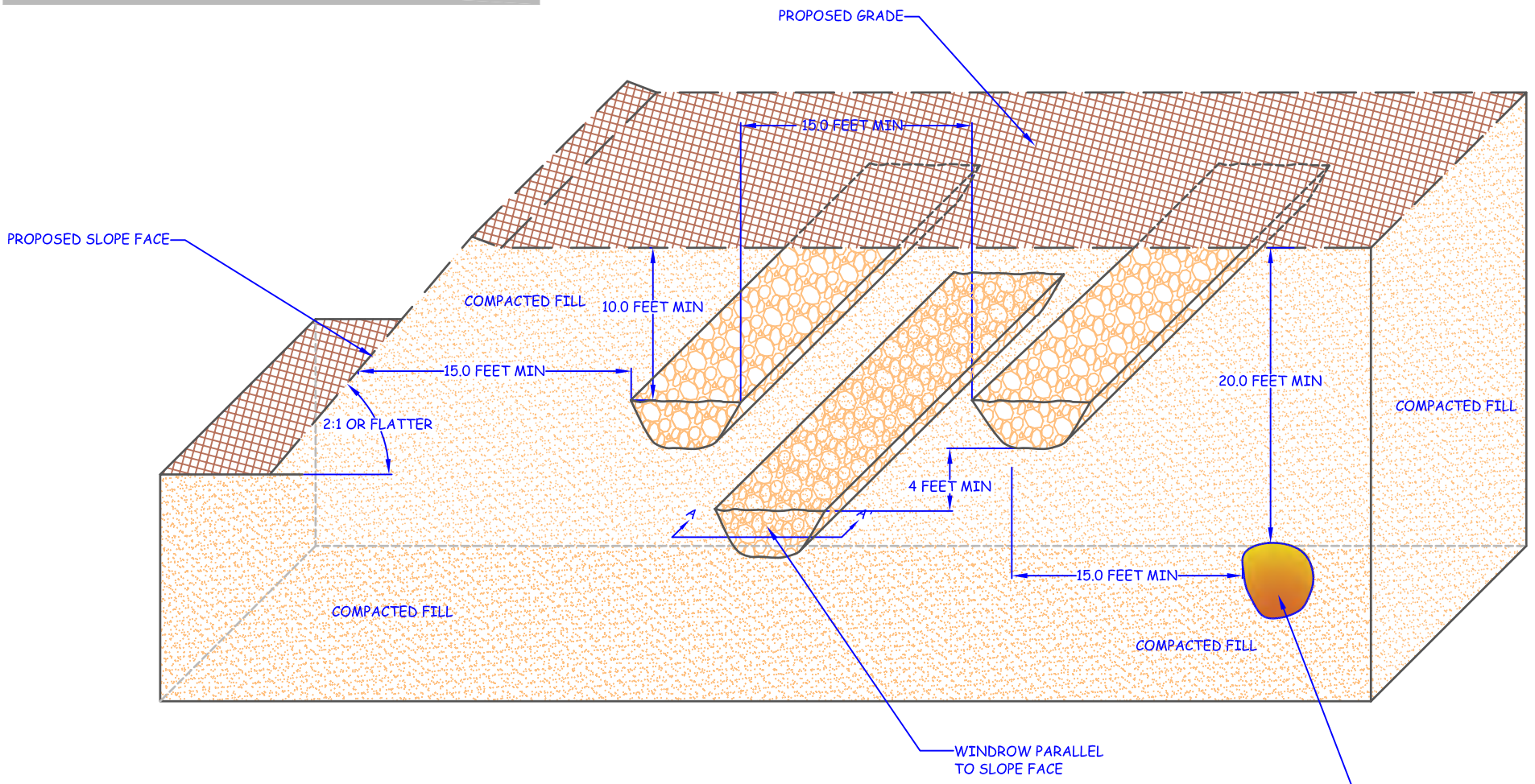
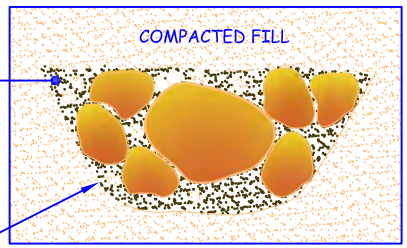
OVERSIZE ROCK TYPICAL DETAIL



CROSS SECTION A-A'

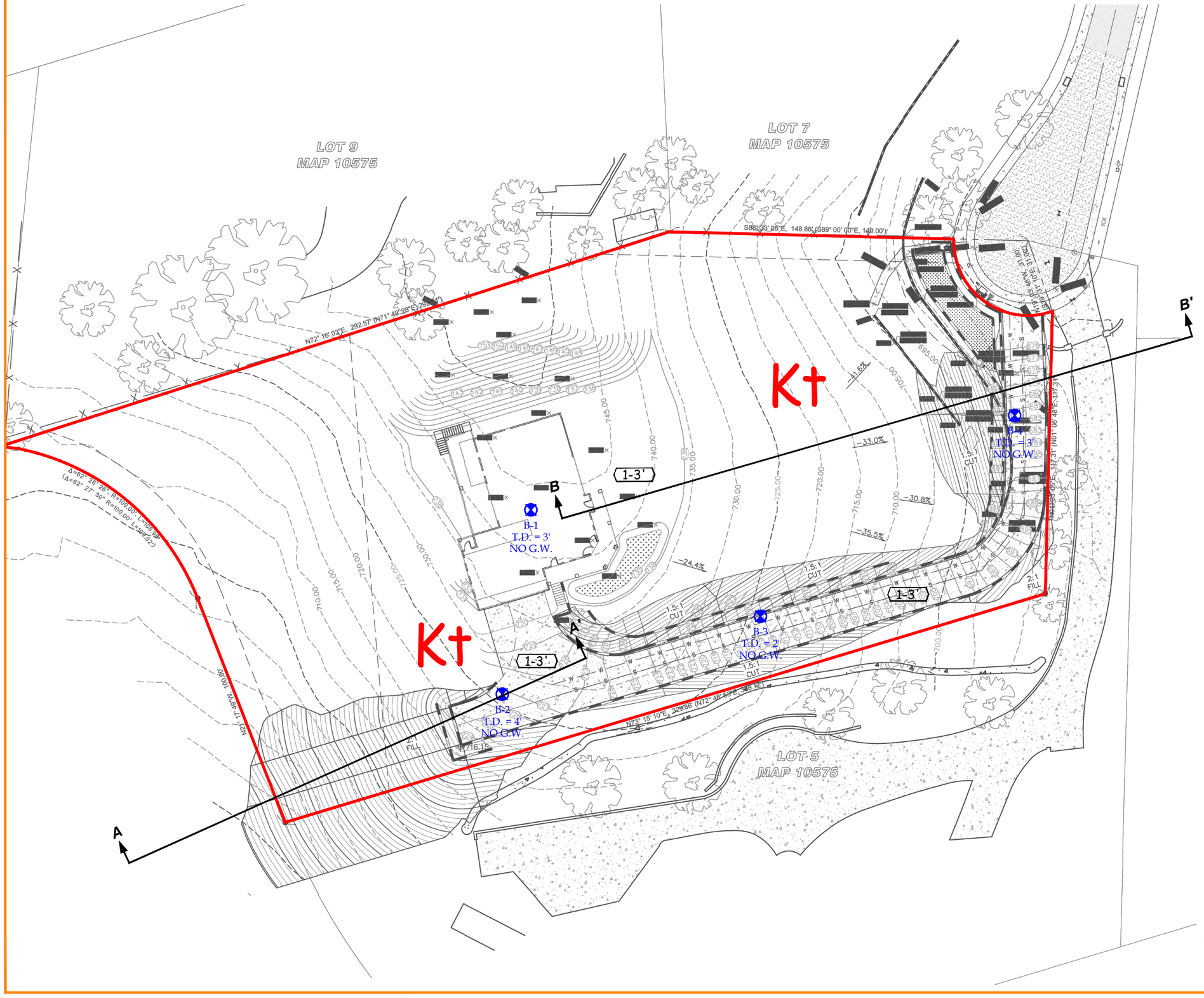


JETTING OF APPROVED
GRANULAR MATERIAL

EXCAVATED TRENCH
OR DOZER V-CUT

NOTES:

OVERSIZE ROCK IS LARGER THAN
8 INCHES IN MAX DIAMETER



LEGEND

Locations are Approximate

Geologic Units

Kt - Cretaceous Tonalite

Symbols

- Limits of Report
- B-4 T.D. = 3' NO G.W. - Boring Location Including Total Depth and Depth to Groundwater
- 1-3' - Recommended Removal Depths
- B — B' - Cross Section Location

NORTH

0 20 40 80

SCALE: 1" = 40'

GEOTECHNICAL MAP

LOCATED ON STONEPOINTE DRIVE

CITY OF ESCONDIDO, SAN DIEGO COUNTY, CALIFORNIA

APN 292-690-06-00

PROJECT	***PROPOSED SINGLE FAMILY RESIDENCE***		
CLIENT	***MR. DAVID HENSEL***		
PROJECT NO.	***256070-10A***		
DATE	***MAY 2025***		
SCALE	***1" = 40'***		
DWG XREFS			
REVISION			
DRAWN BY	***JDG***	***PLATE***	***1 OF 1***

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